Agreement No. CE 92/2017 (CE)

Site Formation and Infrastructure Works for Public Housing Development near Tan Kwai Tsuen, Yuen Long – Investigation, Design and Construction

FINAL WATERWORKS
IMPACT ASSESSMENT
REPORT FOR S16 PLANNING
APPLICATION
(INTENSIFICATION SCHEME)

199086/BIN/089/Issue 3
JULY 2023





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Final Waterworks Impact Assessment Report for S16 Planning Application (Intensification Scheme)

199086/BIN/089/Issue 3

July 2023

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#### 1 INTRODUCTION

#### 1.1 General

- 1.1.1 As a prevailing policy to increase land supply to meet the housing demand in the short, medium and long terms, the Government has identified sites in various districts with the potential to be developed for residential use. Amongst others, a site near Tan Kwai Tsuen (the Site), Yuen Long for public housing developments. The location of the Site is shown on *Drawing No. 199086/BIN/GEN/001*.
- 1.1.2 In view of the acute shortage of housing, the domestic plot ratio of the Application Site is proposed to be intensified to 6.5 with an aim to increase flat production. The Application Site will provide a total of 7,420 public housing units with planned population intake from 2030 by phases. The site formation layout plan is shown on *Drawing No. 199086/BIN/SFAI/001*. An "Application for Permission under Section 16 of the Town Planning Ordinance" is being prepared for the Proposed Development in order to obtain planning permission from the Town Planning Board for minor relaxation of the following restrictions:
  - Maximum plot ratios:
    - Phase 1: from 6.5 to 7.0 (i.e. domestic PR of 6.5 and non-domestic PR of 0.5)
    - Phase 2: from 6.5 to 7.2 (i.e. domestic PR of 6.5 and non-domestic PR of 0.7)
    - Phase 3: from 6.5 to 7.3 (i.e. domestic PR of 6.5 and non-domestic PR of 0.8)
  - Maximum building heights:
    - o Phase 1: from 205 mPD to 240 mPD
    - Phases 2 and 3: from 205 mPD to 235 mPD

#### 1.2 Project Description

1.2.1 Binnies Hong Kong Limited was requested by the Civil Engineering and Development Department (CEDD) to prepare necessary technical assessments of Section 16 (S16) planning application for minor relaxation of PR and building height restriction for the agreement of the Town Planning Board (TPB).

#### 1.3 Interfacing Projects

- 1.3.1 Notable potential interfacing projects include:
- CE 2/2011 (CE) Hung Shui Kiu (HSK) New Development Area (NDA) Planning and Engineering Study
- CE 19/2015 (TP) Preliminary Land Use Study for Lam Tei Quarry and the Adjoining Areas – Feasibility Study



- CE 35/2012 (CE) Planning and Engineering Study for Housing Sites in Yuen Long South – Investigation
- CE 32/2017 (CE) Yuen Long South Development Stage 1 Design and Construction
- PWP Item 7259RS and 7279RS Cycle Tracks connecting North West New Territories with North East New Territories
- CE 46/2007 (DS) Review of Drainage Master Plans (DMP) in Yuen Long and North Districts - Feasibility Study
- PWP Item 4223DS Yuen Long and Kam Tin sewerage treatment upgrade Upgrading of San Wai Sewage Treatment Works
- CE 26/2015 (CE) Site Formation and infrastructural works for the Development at Long Bin, Yuen Long – Feasibility Study
- CE 56/2016 (CE) HSK NDA Stage 1 Works Design and Construction
- CE 39/2016 (CE) HSK NDA Advance Works, Phases 1 & 2 Design and Construction
- CE 42/2016 (CE) Environmental Friendly Transport Services in HSK NDA and Adjacent Areas – Feasibility Study
- CE 86/2017 (CE) Fostering a Pedestrian and Bicycle-friendly environment in HSK
   NDA and YLS Development Feasibility Study
- CE 41/2015 (GE) Landslip Prevention and Mitigation Programme, 2015, Package
   G Landslip Prevention and Mitigation Works and Provision of Emergency Works
   Services for Natural Terrain Landslides Occurring in Mainland West (North) –
   Investigation, Design and Construction
- CE 51/2016 (HY) Route 11 (between North Lantau and Yuen Long) Feasibility Study
- CE 65/2016 (CE) Further Study on Tuen Mun Western Bypass Investigation
- CE 39/2018 (WS) Strategic Cavern Areas to Accommodate Existing and Proposed Service Reservoirs in Lam Tei and Adjoining Areas – Feasibility Study
- CE 6/2019 (DS) Hung Shui Kiu Effluent Polishing Plant and Yuen Long South Effluent Polishing Plant – Investigation
- CE 1/2020 (CE) Hung Shui Kiu/Ha Tsuen New Development Area Package A
   Works for Second Phase Development Design and Construction
- CEDD's project "Greening Master Plan"
- EDB/ ArchSD's primary school projects
- Relocation of Existing Services Reservoirs to Cavern and Proposed Service Reservoirs in Cavern in Lam Tei – Feasibility Study by Project Management Division of WSD

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- Holistic Review Study on the Use of Reclaimed Water in Northwest New Territories by West Development Office of CEDD
- Other utilities supply to the Development
- Other relevant projects of planned/committed developments as advised by PlanD, HKHA/ HD, CEDD and other Government B/Ds
- Any other relevant projects and assignments, which may arise during the course of the Assignment.

#### 1.4 Purposes and Scope of this Report

1.4.1 The purpose of this report is to evaluate the potential waterworks impacts associated with the proposed housing development due to the proposed increase in maximum plot ratio and building height.

#### 1.5 Structure of the Report

- 1.5.1 This report comprises the following sections: -
  - **Section 1** introduces the project background and project description.
  - **Section 2** describes the methodology ad design criteria for the report.
  - **Section 3** presents the existing and planned waterworks facilities around the Site.
  - **Section 4** conducts the water demand estimate of fresh water and salt water supplies by the Site.
  - **Section 5** assesses the potential impact to waterworks facilities arising from the Site.
  - **Section 6** proposes the mitigation and water supplies schemes due to the Site.
  - **Section 7** presents the preliminary design of the water supply system.
  - **Section 8** concludes the findings and makes recommendations.

#### 2 METHODOLOGY AND DESIGN CRITERIA

#### 2.1 Information Available for WIA Study

- 2.1.1 To conduct the study on the waterworks, we have adopted the following available information:
  - Existing water mains records provided by WSD;
  - As-built drawings on existing waterworks; and
  - WSD's Planning Report.

#### 2.2 Methodology

- 2.2.1 The Development will generate water demands of fresh and flushing waters. The expected impact on the existing water supplies system and their requirement for any upgrading works or new facilities to meet such demands and the correspondence implementation programme, if necessary, will be discussed in this report.
- 2.2.2 Coordination with Water Supplies Department (WSD) was conducted to acquire relevant information and data, including but not limited to existing capacities of water supplies systems and etc. serving the Site. Meetings with PlanD, CEDD and WSD and other relevant departments will be conducted for further discussion if needed.
- 2.2.3 Under this report, the following approach has been adopted to carry out the study:
  - To identify the scope of the developments at the Site;
  - To determine the water demands of the developments at the Site;
  - To assess the adequacy of the existing and planned water supply facilities within and in the vicinity of the proposed development boundary;
  - To examine the impact arising from additional water demands from the developments at the Site on the existing source of supply and the system capacity; and
  - To propose the required water mains layout.
- 2.2.4 The estimation of the water demands of the Development is based on the latest development parameters provided by Housing Department (HD) and relevant government departments as indicated in *Table 2.2* and are generally with reference to the unit water demands as recommended under WSD's Departmental Instruction (DI) No. 1309 and "Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning" published by EPD.
- 2.2.5 This report has been undertaken in accordance with the following standards, Code of Practice and Design Manuals:
  - WSD's Civil Engineering Design Manual;
  - EPD's Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning;

- WSD's Manual of Mainlaying Practice (2012 Edition); and
- WSD's Departmental Instruction (DI) No. 1309.

#### 2.3 Design Parameters

2.3.1 In accordance with WSD's DI 1309, Review of Planning Standards - Part I: Balancing Storage for Fresh Water Service Reservoir and Review of Planning Standards – Part V: Fire Fighting Water Requirements, and AD/Dev's memo of ref. (8) in WSD 3608/50/3/02 Pt.2 dated 11 May 2006, the following design parameters and peak demand factors are adopted for the design of proposed water supply system for the Development. *Table 2.1* lists the relevant design criteria to be used for the assessment.

Table 2.1 – Design Criteria from WSD's DI 1309 and AD/Dev's memo of ref. (8) in WSD 3608/50/3/02 Pt.2

Waterworks / Facilities	Requirements
Service Reservoir Capacity	<ul> <li>Fresh water system <ul> <li>(i) Supply Zone</li> <li>Interconnected Supply System: 0.75MDD</li> <li>Isolated Supply Zone: 0.85MDD</li> <li>(ii) Fire-fighting Requirements</li> <li>For R1 = [0.4MDD + 9,900/2] / 0.9</li> <li>For R2 = [0.4MDD + 6,600/3] / 0.9</li> <li>For R1 = [0.4MDD + 3,300/4] / 0.9</li> <li>(iii) With Critical Customer</li> <li>Add 0.05MDD Storage Capacity</li> </ul> </li> <li>Flushing water system - 64% of MDD (for reclaimed water)</li> </ul>
Pumping Station Capacity	<ul> <li>Fresh water system         <ul> <li>(i) Pump Rate – 1.5MDD (About 8330 m³/d)</li> </ul> </li> <li>Flushing water system         <ul> <li>(ii) Pump Rate – 1.2MDD (About 1810 m³/d)</li> </ul> </li> </ul>
Peak Flow Rates in Distribution Main	<ul> <li>Fresh water system – 3 times mean daily demand</li> <li>Flushing water system – 2 times mean daily demand</li> </ul>

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Waterworks / Facilities	Requirements	
Residual Head	<ul> <li>Fresh water system – 20m</li> <li>Fresh water system for firefighting – 17m</li> <li>Flushing water system – 15m</li> </ul>	
Fire Fighting	• Fire-fighting requirements for Zone R1 = 9,900 m <sup>3</sup> /d for 12 hours; Zone R2 = 6,600 m <sup>3</sup> /d for 8 hours & Zone R3 = 3,300 m <sup>3</sup> /d for 6 hours.	

#### 2.4 Development Parameters

2.4.1 The latest development parameters of the Development is shown below in *Table 2.2* and *Table 2.3*.

**Table 2.2 - Latest Development Parameters** 

Table 2.2 - Latest Development Parameters				
Domest	ic			
	Plot Ratio	6.5		
	Total No. of Flats	7,420		
Non-do	mestic			
	GFA for Welfare Facilities (m²)	15,849		
	GFA for Retail Complex (m <sup>2</sup> )	5,912		
	GFA for Car Parking (m <sup>2</sup> ) 42,850			
GFA for Ancillary Facilities <sup>(1)</sup> (m <sup>2</sup> ) 1,098				
	GFA for Other Facilities <sup>(1)</sup> (m <sup>2</sup> ) 8,800			
	Total GFA <sup>(2)</sup> (m <sup>2</sup> ) 31,658			
School				
	No. of Kindergarten	2		
	No. of Primary School	1		
Notes:				
(1)	(1) Ancillary facilities and other facilities include office, workshop, PTI and covered			
	walkway etc.			
(2)	Excluding GFA for carparking.			

**Table 2.3 - Design Populations** 

Domestic					
	No. of Population 20,034 <sup>(1)</sup>				
Retail a	nd Welfare				
	No. of Employees <sup>(2)(3)</sup> 1,986				
Primar	y School and Kindergarten				
	No. of students and teachers (4)(5) 1,223				
Notes:					
(1)	Person per flat = 2.7				
(2)	Worker densities of 3.5 workers (Retail Trade) and 3.3	workers (Community, Social &			
	Personal Services) per 100m <sup>2</sup> GFA are adopted based	on Table 8 of Commercial and			
	Industrial Floor Space Utilization Survey published by Planning Department for the				
	estimation of employees in retail and welfare / ancillary facilities, respectively.				
(3)	The provision of welfare facilities is subject to the approval of Social Welfare Department.				
(4)	Two kindergartens was planned with 180 students per kindergarten; 25.5 students per				
	class for primary school are assumed based on Chapter 3 of Hong Kong Planning				
	Standards and Guidelines.				
(5)	Pupil-Teacher ratios of 8.6:1 (kindergarten) and 13.8:1				
	based on Education Bureau's 2017/18 figures and statistics available on Education				
	Bureau's website.				

2.4.2 The latest Preliminary Layout Plan of the Development at the Site provided by HD is enclosed in *Appendix E* for reference.

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#### 3 EXISTING AND PLANNED WATERWORKS FACILITIES

#### 3.1 Existing Water Treatment Works

3.1.1 Au Tau Water Treatment Works (ATWTW) is the current major water source to the Tan Kwai Tsuen area. Treated fresh water is transferred from ATWTW via Au Tau Primary Service Reservoir to Tan Kwai Tsuen South Fresh Water Service Reservoir (TKTS FWSR).

#### 3.2 Existing Service Reservoirs

#### Fresh Water Service Reservoirs

- 3.2.1 The Site is located adjacent to the TKTS FWSR which supplies fresh water to Tan Kwai Tsuen and the surrounding areas.
- 3.2.2 The Site is located within the fresh water supply zone of the existing TKTS FWSR. The above key waterworks and fresh water service reservoir supply zones are shown in *Appendix A*.
- 3.2.3 *Table 3.1* summarizes the information on the capacity, top water level and invert level of the existing TKTS FWSR.

Table 3.1 – Information on Existing Tan Kwai Tsuen South Fresh Water Service Reservoir

Service Reservoir	Capacity (m³)	Top Water Level (mPD)	Invert Level (mPD)
Tan Kwai Tsuen South FWSR	77,742 (1)	67.00	60.00

Notes:

#### Salt Water Service Reservoirs

- 3.2.4 The Site is located in close proximity of the salt water supply zone of the existing Tan Kwai Tsuen Salt Water Service Reservoir (TKT SWSR) which is sourced from Lok On Pai Salt Water Pumping Station (LOP SWPS). The above key waterworks and salt water service reservoir supply zones are shown in *Appendix B*.
- 3.2.5 *Table 3.2* summarizes the information on the capacity, top water level and invert level of the existing TKT SWSR.

Table 3.2 – Information on Existing Tan Kwai Tsuen Salt Water Service Reservoir

Service Reservoir	Capacity (m³)	Top Water Level (mPD)	Invert Level (mPD)
Tan Kwai Tsuen SWSR	18,100(1)	67.50	60.00

Notes:

<sup>(1)</sup> The capacity of the service reservoirs is obtained from the record plans provided by WSD.

<sup>(1)</sup> The capacity of the service reservoirs is obtained from the record plans provided by WSD.

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#### 3.3 Existing Water Mains

#### Fresh Water Supply

- 3.3.1 With reference to WSD's mains record plans (MRPs), the alignment of the existing fresh water mains in the vicinity of the Site are shown in *Drawing Nos.* 199086/BIN/WIA/002 to 005. The existing fresh water supply services in the vicinity of the Site are described as follows:
- 3.3.2 There are three existing fresh water mains, including DN1400 and DN1800 trunk mains and DN150 pipe, running along the future alignment of the northern section of the proposed road. The interface between the DN1400 and DN1800 trunk water mains and the proposed road ends near the north tunnel portal of the WSD tunnel, while the DN150 pipe continues to run along the future alignment of the proposed road.
- 3.3.3 There are existing DN50 to DN80 galvanized iron pipes serving the cottages within the Site. A DN150 ductile iron fire main is also located along Yuen Long Highway at the west of the Site connecting the fire hydrants on the carriageway.
- 3.3.4 There are three existing fresh water mains, including DN300, DN150 and DN1400 fresh water mains, along the future alignment of the proposed road near the North West N.T. Refuse Transfer Station.

#### Salt Water Supply

- 3.3.5 With reference to the WSD's MRPs, the existing salt water mains in the vicinity of the Site are shown in *Drawing Nos. 199086/BIN/WIA/006 to 008*. The existing salt water supply services in the vicinity of the Site are described as follows:
- 3.3.6 A section of DN1000 trunk salt water main is identified crossing the future alignment of the proposed road at Shui Fu Road connecting to Tan Kwai Tsuen Salt Water Service Reservoir.
- 3.3.7 No existing salt water main is identified within the Site.

#### 3.4 Planned Waterworks Facilities

#### **Planned Waterworks**

3.4.1 The interfacing projects are summarized in *Table 3.3* as follows:

#### Table 3.3 - Summary of Interfacing Projects and Planned Waterworks

#### Projects/ Planned Waterworks

- 1. CE 39/2018 (WS) Strategic Cavern Areas to Accommodate Existing and Proposed Service Reservoirs in Lam Tei and Adjoining Areas Feasibility Study
- 2. CE 56/2016 (CE) HSK NDA Stage 1 Works Design and Construction
- 3. CE 39/2016 (CE) HSK NDA Advance Works, Phase 1 & 2 Design and Construction
- 4. CE 35/2012 (CE) Planning and Engineering Study for Housing Sites in Yuen Long South Investigation
- 5. CE 71/2020 (CE) Hung Shui Kiu/Ha Tsuen New Development Area Package B Works for Second Phase Development Design and Construction

#### Projects/ Planned Waterworks

- 6. CE 78/2020 (WS) Ngau Tam Mei WTW PSR Ext IDC
- 7. CE 1/2020 (CE) "Hung Shui Kiu/Ha Tsuen New Development Area Package A Works for Second Phase Development Design and Construction"

#### Planned Fresh Water Supply System

- 3.4.2 According to the advice of WSD, the feasibility study for relocating the proposed service reservoirs and existing service reservoirs in Lam Tei and adjoining areas to strategic cavern areas has just been awarded in July 2019. Close liaisons with WSD will be carried out to address the interface issues.
- 3.4.3 There are also potential interfaces with the water supply systems proposed under HSK NDA and Yuen Long South (YLS) Development. Close liaisons with the project offices of HSK NDA and YLS Development projects will be conducted to address the interface issues.

#### Planned Salt/Flushing Water Supply System

3.4.4 In the coordination meeting held on 18 February 2019 with the project team of the YLS Development Project and PlanD, CEDD/WDO advised the latest arrangement on the storage and reuse of reclaimed water from the planned YLS Effluent Polishing Plant (EPP). The reclaimed water from the YLS EPP would be conveyed and stored in a proposed service reservoir named Tan Kwai Tsuen East Reclaimed Water Service Reservoir (TKT RWSR) in the vicinity of the TKT Development for serving the HSK NDA and the surrounding areas. Close liaisons with HSK NDA project team to confirm the location of branching off their proposed reclaimed water main is underway.



# 4 ESTIMATED WATER DEMAND AND VOLUME OF PROPOSED SERVICE RESERVOIRS FOR THE DEVELOPMENT

#### 4.1 Design Parameter

4.1.1 With reference to WSD's DI No. 1309 and EPD's Guidelines for Estimating Sewage Flows for Sewage Infrastructure Planning, WSD's consumer classifications, fresh water and flushing water unit demands as shown in *Table 4.1* were assumed for the estimation of water demands of the Development.

Table 4.1 - Unit Demands Assumed for the Development

Unit Demand	The Site (for PRH & SSF)	School	Non-domestic (3)		
WSD's Consumer Classification	R1	School <sup>(1)</sup>	Retail	Welfare Facilities	Ancillary Facilities and Others
Fresh Water Unit Demand (l/h/d)	230	25	-	200	40
Service Trade Unit Demand (2) (1/h/d)	40	-	-	-	-
Flushing Water Unit Demand (l/h/d)	70	30	70	70	70

#### Notes:

- (1) A 30-classroom of primary school has been proposed
- (2) In accordance with Table 2 of Departmental Instruction No. 1309 issued by WSD
- (3) Calculated from data in Guidelines for Estimating Sewage Flows issued by EPD

#### 4.2 Estimated Water Demand

4.2.1 According to the development parameters as advised in paragraph 2.4.1, the water demands and the required capacity of the proposed service reservoirs for the Development are summarized in *Table 4.2* and *Table 4.3* below respectively. Detailed calculation is annexed in *Appendix C*.

Table 4.2 - Summary of Water Demands

Anticipated	Fresh Water	Flushing	Peak Fresh	Peak Flushing	
Population	MDD	Water MDD	Water MDD	Water MDD	
(h)	(m <sup>3</sup> /day)	(m <sup>3</sup> /day)	(m <sup>3</sup> /day)	(m <sup>3</sup> /day)	
<b>19,575</b>	<mark>5,554</mark>	<mark>1,508</mark>	<mark>17,554</mark>	<mark>3,016</mark>	

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**Table 4.3 - Summary of Water Service Reservoir** 

	Fresh water reservoir	Flushing water reservoir
Required Capacity (m <sup>3</sup> )	<mark>7,970</mark>	<mark>969</mark>
Design Capacity (m <sup>3</sup> )	8,100	1,000
TWL(mPD)	<mark>123</mark>	<mark>113.5</mark>
IL(mPD)	115	110

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#### 5 POTENTIAL IMPACT TO EXISTING AND PLANNED WATERWORKS

### 5.1 Impact on Existing Water Treatment Works and Existing Water Service Reservoirs

Fresh Water Supply

- 5.1.1 The Site is located near Tan Kwai Tsuen in Yuen Long which is within the fresh water supply zone of the existing ATWTW. As such, it is considered that there will be no adverse impact on the current zoning of the ATWTW due to development of the Site.
- 5.1.2 The existing TKTS FWSR is at +60mPD while the proposed public housing development will be formed at +42mPD to +82mPD. Direct supplying water from the existing TKTS FWSR is not feasible due to insufficient head.
- 5.1.3 Mitigation measures to resolve the residual head problem is necessary which will be discussed in Section 6.

#### **Flushing Water Supply**

- 5.1.4 The existing capacity of LOP SWPS does not cater for the flushing demand of the proposed public housing development at the Site.
- 5.1.5 In view of the above, mitigation measures are required and details are presented in Section 6.

#### 5.2 Impact on Existing and Planned Water Mains

#### Within the Site

5.2.1 As mentioned in Section 3.3, there are some existing fresh water mains within or in vicinity of the Site. It is anticipated that only minor diversion/relocation/removal to these existing mains (e.g. the galvanised iron FW mains connecting the cottages at the western part of the Site) will be required, and hence there will be no major impact to the existing water mains.

#### Outside the Site

- 5.2.2 A WSD tunnel and the associated tunnel portals and no-blasting zone are identified to the north of the Site. An access road will be constructed to connect the Site to the Tin Shui Wai West Interchange. No blasting would be carried out within the 120m no-blasting zone for the construction of this access road to avoid affecting the WSD tunnel.
- 5.2.3 It is anticipated that diversions of existing water mains are required for the construction of access road from the Site to the Tin Shui Wai West Interchange and Shun Tat Street. The sections of existing fresh water mains with potential conflicts are shown in *Drawing Nos.* 199086/BIN/WIA/009 to 012, while preliminary diversion plan is enclosed in *Appendix D*. The sections of existing salt water mains with potential conflicts are shown in *Drawing No.* 199086/BIN/WIA/013. Detailed design of the diversion will be proposed in the detailed design stage of the project.
- 5.2.4 In addition, the existing DN300 ductile iron washout pipe will be in conflict with the proposed pumping station and is proposed to be relocated. The detailed relocation

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scheme will depend on findings of the utility survey and be proposed in the later stage of the project.

#### 5.3 Water Gathering Ground

5.3.1 According to WSD's record drawings on water gathering ground enclosed in *Appendix F*, the Site does not fall into WSD's Water Gathering Ground and hence both short-term and long-term impacts on the Water Gathering Ground are not anticipated.

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#### 6 PROPOSED WATER SUPPLIES SCHEMES AND MITIGATION PROPOSALS

#### 6.1 Proposed Fresh Water Supply and Fresh Water Service Reservoir

- 6.1.1 Considering that the Site is located adjacent to the fresh water supply zone of TKTS FWSR, it is proposed to incorporate the Site into the above zone to avoid substantial rezoning arrangement.
- 6.1.2 The existing TKTS FWSR is at +60mPD while the proposed public housing development will be formed at +42mPD to +82mPD. Direct supplying water from the existing TKTS FWSR is not feasible due to insufficient head. As such, a high-level fresh water service reservoir is required. Fresh water will be sourced from the existing TKTS FWSR and pumped to the proposed high-level fresh water service reservoir via a booster pumping station.
- 6.1.3 A high-level fresh water service reservoir to be located at +115mPD near the Site will be provided to accommodate the fresh water demand of the Development. An access road will be provided to facilitate the operation and maintenance of the above waterworks facilities.
- 6.1.4 According to the Review of Planning Standards Part I Storage Capacity for Fresh Water Service Reservoir published by WSD, the capacity of the proposed fresh water service reservoir shall be calculated in accordance with WSD's Review of the Prevailing Planning Standards for DI 1309, including scenarios of isolated supply zone and fire-fighting requirement. The calculation for the capacity of the proposed fresh water service reservoir is presented in *Appendix C*.
- 6.1.5 As presented in *Appendix C*, the proposed public housing development and its ancillary facilities will require mean daily demand of fresh water of about 5,554m³/day to accommodate the anticipated fresh water consumption in domestic, non-domestic and service trade uses.
- 6.1.6 Based on the water demand forecast, the proposed fresh water service reservoir will have a design capacity of about 8,100m<sup>3</sup> and will be located at +115mPD with invert level also at +115mPD to meet the water demand of the Development and provide sufficient residual head.
- 6.1.7 The location and level of the proposed fresh water supply facilities are shown in *Drawing Nos.* 199086/BIN/WIA/014 to 017.
- 6.1.8 As the Project had been included in Category C of the Public Works Programme subsequent to the approval of its Technical Feasibility Statement in August 2017, DEVB TC(W) No. 8/2017 with the effective date on 22 Dec 2017 should not be applicable. Nevertheless, reasons for not constructing the proposed service reservoirs in caverns under this Project are explained in the following paragraphs.
- 6.1.9 In order to meet the housing demand, the population intake of this Project was planned to be from 2030 by phases. In view that more than 10 years lead time is necessary for constructing reservoirs in cavern, the proposed fresh water and flushing water service reservoirs would be necessary to be constructed in open areas

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instead of inside caverns to cater water demand arisen from the Development before the planned population intake.

#### 6.2 Proposed Flushing Water Supply and Flushing Water Service Reservoir

#### Interim Scenario

- 6.2.1 With reference to the latest tentative implementation programme of YLS Development, the planned Tan Kwai Tsuen Reclaimed Water Service Reservoir (TKT RWSR) will be commissioned in 2033 while the proposed public housing development under this Assignment will have population intake from 2030 by phase. Programme mismatch exists between the commissioning of the reclaimed water supply facilities and the population intake of the Development.
- 6.2.2 The design capacity of LOP SWPS is 83MLD for delivering salt water to areas including Tin Shui Wai and Yuen Long Town. When the supply of reclaimed water to be available later, we understand that WSD will switch the FLW supply from salt water to reclaimed water for areas including Tin Shui Wai and Yuen Long Town so that the loading on LOP SWPS will be reduced significantly. In this connection, the existing pumps in LOP SWPS for the proposed development may only upgrade if the progress of reclaimed water supply for Tin Shui Wai and Yuen Long Town would be delayed.
- 6.2.3 With the expected availability of salt water supply from LOP SWPS, salt water supply from TKT SWSR is proposed as the interim flushing water supply before reclaimed water will be available.
- 6.2.4 As reclaimed water is the ultimate flushing source for the proposed public housing development, a high-level flushing water service reservoir is proposed at +110mPD with invert level also at +110mPD. The design capacity is 1,000m³ to supply reclaimed water with sufficient residual head to the proposed housing sites. Before reclaimed water is available, salt water will be supplied by the existing TKT SWSR to the proposed high-level flushing water service reservoir via a booster pumping station.
- 6.2.5 Storage requirement of the proposed flushing water service reservoir is controlled by the ultimate scenario which adopt reclaimed water for flushing water supply as discussed in the following paragraphs.
- 6.2.6 The proposed interim flushing water supply arrangement is shown in *Drawing Nos.* 199086/BIN/WIA/018 to 020.

#### <u>Ultimate Scenario</u>

- 6.2.7 With reference to the Holistic Review Study on the Use of Reclaimed Water for Flushing in Northwest New Territories, the flushing water for the Development is proposed to be supplied by reclaimed water from the planned YLS EPP via the planned TKT RWSR considering the cost-benefit.
- 6.2.8 Derivation of the required capacity needed for the proposed flushing water service reservoir is presented in *Appendix C*. The total anticipated flushing water MDD required for domestic and non-domestic uses of the proposed public housing

- development would be  $1,508 \text{ m}^3/\text{day}$ . The design capacity of the proposed high-level flushing water service reservoir would be  $1,000 \text{ m}^3$  (i.e. 1,508 x 0.64) <sup>1</sup>.
- 6.2.9 It is understood that further coordination with WSD and the project teams of YLS Development project, HSK NDA project and YLS EPP project are required to ascertain the detailed arrangement of reclaimed water supply.
- 6.2.10 Locations and levels of the proposed ultimate flushing water supply facilities are shown in *Drawing Nos.* 199086/BIN/WIA/021 to 023.

#### 6.3 Proposed Distribution Mains

Proposed Fresh Water Mains (FWM)

- 6.3.1 It is proposed to source the Development from the proposed fresh water service reservoir as detailed in Section 6.1. Fresh water mains of size DN450 to DN300 are proposed to be laid.
- 6.3.2 The proposed network of fresh water mains connecting the proposed fresh water service reservoir with the proposed pumping station and the Development will be discussed in Section 7.2.
- 6.3.3 To enhance the reliability of the proposed flushing water supply system, a 200mm diameter fresh water augmentation main will be laid branching off from the inlet main of the proposed high-level fresh water service reservoir to the proposed flushing water service reservoir as an alternative supply source for flushing supply to the proposed housing development in the event that the proposed flushing water pumping station has to be shut down due to operational reasons or as a result of other unforeseeable events. The invert level of the concerned fresh water main should be set at a level higher than the crown level of the overflow pipe to avoid accidental contamination of the fresh water supply system by flushing water.

#### **Proposed Flushing Water Mains**

Interim scenario

- 6.3.4 It is proposed to supply the Development by the existing TKT SWSR via the proposed booster pumping station as detailed in Section 6.2. Flushing water mains of size DN80 to DN200 are proposed to be laid. The proposed flushing water mains should meet the requirement of conveying reclaimed water such that it could be easily converted to reclaimed water in the future.
- 6.3.5 The proposed network of flushing water mains connecting the proposed flushing water service reservoir with the proposed pumping station and the Development will be discussed in Section 7.3.

Ultimate scenario

6.3.6 It is proposed to supply the Development by the proposed flushing water service reservoir to be fed by the proposed reclaimed water from YLS Development via the

<sup>&</sup>lt;sup>1</sup> The storage requirement is similar to the requirement of proposed treated grey water service reservoir for flushing under Anderson Road Quarry development. The capacity of treated grey water service reservoir for flushing is 64% of the MDD, which consists of 25% for balancing, 33% for breakdown and allow 10% capacity as ineffective storage (i.e. [25+33]/0.9% of MDD).



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- proposed pumping station as detailed in Section 6.2. The proposed DN200 flushing water mains laid for the interim scenario would be used for supplying reclaimed water and thus no additional water mains would be laid.
- 6.3.7 In order to minimize the risk of cross-connections between the fresh water supply system and reclaimed water supply system, the flushing water pipes with reclaimed water should have distinctive features different to those pipes used for the fresh water supply system including but not limited to the use of purple pipe coating, labelling and different pipe sizes.

#### **6.4 Proposed Firefighting Water Supply**

6.4.1 The locations of fire hydrant and corresponding distribution mains are shown in the *Drawing Nos.* 199086/BIN/WIA/014 to 017.

Along the proposed access road

- 6.4.2 Distribution mains of size DN 300 supplying fresh water is proposed to be laid with fire hydrants staggered every 300m along alternate sides of the proposed access road to be connected to Tin Shui Wai West Interchange (TSWWI) slip road and Shun Tat Road, as well as the access road between housing platforms.
- 6.4.3 The above distribution mains will be connected to the proposed DN450 fresh water main to be fed by the proposed fresh water service reservoir.
  - Within the proposed housing sites
- 6.4.4 Sufficient water capacity for firefighting has been reserved in the proposed fresh water service reservoir and the associated fresh water mains.
- 6.4.5 Firefighting water supply for the Development site will be provided by the fresh water mains along the proposed public access road between the housing platforms. Exact position of fire hydrants within the Development site should be determined together with the detailed layout design of the by the relevant departments.

#### 6.5 Proposed Pumping Station

- 6.5.1 As aforementioned, two high-level reservoirs for supplying fresh water and flushing water would be required to support the Development. In view of the elevation difference between the existing TKT SWSR and the proposed flushing water service reservoir (for interim scenario of flushing water supply); the planned TKT RWSR and the proposed flushing water service reservoir (for ultimate scenario of flushing water supply); the existing TKTS FWSR and the proposed fresh water service reservoir, pumping facilities are required to provide sufficient pressure to feed the fresh water and flushing water to the proposed service reservoirs.
- 6.5.2 To minimize the footprint of the pumping station, the proposed fresh water and flushing water pumping stations are proposed to be combined and co-located at the level of about +52.5mPD.
- 6.5.3 A single-storey pumping station with headroom of about 4m to 7m is proposed to be located near the south side of the junction between Shui Fu Road and the proposed

- access road, which is bounded by the existing drainage channel to the south and existing trunk water mains to the east.
- 6.5.4 The inlet for fresh water supply is proposed to be branched off from the existing DN1000 mild steel fresh water mains (FWM), located on the east of the proposed pumping station near Shui Fu Road. While as the outlet is proposed to be emanated from the north of the pumping station. Fresh water will be pumped along the existing WSD's access road and further along the proposed access road to reach the proposed fresh water service reservoir.
- 6.5.5 The inlet for flushing water supply is proposed to be branched off from the existing DN1000 salt water mains (SWM) at the junction between Shui Fu Road and the proposed access road. Both inlet and outlet are proposed at the north side of the pumping station in the interim stage to facilitate the future switching process from the use of salt water for interim stage to the use reclaimed water for the ultimate scenario. Similar to the fresh water supplies, flushing water would be pumped along the existing WSD's access road and further along the proposed access road to reach the proposed flushing water service reservoir.
- 6.5.6 Various auxiliary rooms including switch rooms, control room, E&M plant room are proposed in the pumping station. The location and preliminary layout of the pumping station is indicated in *Drawing No. 199086/BIN/WIA/024*.

#### **Pumps**

- 6.5.7 Kinetics pump in the form of centrifugal pumps are proposed due to the small scale in nature. To cater for emergency situation and facilitating the maintenance, two pumps (one duty and one standby) sizing at peak flow are proposed for both flushing water and fresh water. The layout of the pumping station will be reviewed in detailed design stage to determine if space can be reserved for an additional pump for future upgrade.
- 6.5.8 Automatic pump operation system based on the water level in the service reservoir is recommended. Level sensing equipment will be provided for the purpose of pump control and emergency alarm. The standby pump will cut in when the duty pump fails to operate.

#### **Electricity Power Supply**

6.5.9 Dual feed power supply is proposed to enhance the stability of the power supply. CLP Power (CLPP) will be liaised for providing the dual feed system.

#### Ventilation and Air-conditioning

- 6.5.10 Apart from the control room, ventilation will be provided to all the remaining plant rooms in order to maintain and limit the temperature rise inside the pumping station such that the maximum ambient temperature in pump hall will not exceed 40°C.
- 6.5.11 The fresh air inlets and exhaust air outlets will be at the lowest and the highest possible levels respectively. Air inlets, outlets and ducts, if any, will be arranged to minimize stagnant air pockets inside the plant room and ensure no exhaust air is re-

- circulated. Fixed louvres with removable wire mesh screens will be provided for the air inlets and outlets.
- 6.5.12 Independent compact air-conditioning units will be installed in the control room in accordance with WSD current practice.
- 6.5.13 The ventilation system will be designed to comply with ASHRAE and all equipment will comply with relevant British Standards.

#### Monitoring and Control

6.5.14 SCADA/PLC system will be provided for the electrical equipment and the pumping system with adequate input and output (I/O) points. Hot standby PLC shall be provided with redundant configuration for a safe, secure and trouble–free condition in the event of any malfunction in a PLC in the network.

#### 6.6 Proposed District Metering Areas and Pressure Management Areas

6.6.1 District Metering Areas (DMA) and Pressure Management Areas (PMA) are proposed in accordance with the clauses 1.2.4 / 1.2.5 of the Manual of Mainlaying Practice (2012 Edition). The DMA and PMA for proposed fresh and flushing water service reservoir are indicated in *Drawing No.* 199086/BIN/WIA/025 and 026 respectively.

#### 6.7 Smart Water Initiatives

- 6.7.1 The adoption of Automatic Meter Reading and the aspect of smart initiatives will be taken into consideration in the design stage. Development (1) Division of WSD will be consulted on smart initiatives during design stage, including but not limited to:
  - Intensive metering and real-time monitoring for Water Intelligent Network (WIN);
  - Smart/Intelligent pressure management;
  - Hydraulic modelling for leak pin-pointing and network operation;
  - Water suspension notification system;
  - On-line water quality monitoring at the distribution networks; and
  - Leak pin-pointing with fibre optic technology.

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#### 7 PRELIMINARY HYDRAULIC DESIGN OF PROPOSED DISTRIBUTION MAINS

#### 7.1 Hydraulic Analysis

7.1.1 It is proposed to feed the fresh water and flushing water to the proposed Development site by laying water mains to connect the proposed water service reservoirs and the existing water supply networks. *Tables 7.1* and *7.2* below summarize the diameters and capacities of the proposed water mains. It is revealed that all the proposed pipes will have sufficient capacities for the peak flows.

**Table 7.1 - Summary of Proposed Fresh Water Mains Capacities** 

Size of Main (mm)	Available Peak Flow Velocity (m/s)	Required Peak Flow Velocity (m/s)	Available Peak Flow Capacity (m³/day)	Required Peak Flow Capacity (m³/day)
450	2	1.28	<b>27483</b>	17,534
300	1.5	0.99	9,161	6,020
250	1.5	1.42	6,362	6,020

**Table 7.2 - Summary of Proposed Flushing Water Mains Capacities** 

Size of Main (mm)	Available Peak Flow Velocity (m/s)	Required Peak Flow Velocity (m/s)	Available Peak Flow Capacity (m³/day)	Required Peak Flow Capacity (m³/day)
200	1.5	<mark>1.11</mark>	4,072	<mark>3,016</mark>
100	1.5	1.49	1018	1,008
<mark>80</mark>	1.5	<mark>0.12</mark>	<mark>651</mark>	<mark>50</mark>

7.1.2 Preliminary hydraulic calculations have been carried out for the proposed water supply systems. It is noted that minimum residual heads of the proposed fresh water system and flushing water system fulfill the WSD's requirements. The calculations are annexed in *Appendix C*.

#### 7.2 Preliminary Design - Fresh Water

**Inlet of the Proposed Pumping Station** 

7.2.1 DN300 FWM is proposed to be branched off from the existing DN1000 FWM before inlet to the existing Tan Kwai Tsuen Fresh Water Service Reservoir (SR216) and to be connected to the inlet of the proposed pumping station.

Outlet of the Proposed Pumping Station and Inlet of the Proposed FWSR

7.2.2 DN300 FWM is proposed as the outlet of the proposed pumping station and to be laid along the existing and proposed WSD's maintenance access road to feed fresh water to the proposed fresh water service reservoir.

Outlet of the Proposed FWSR

7.2.3 DN450 FWM is proposed as the outlet of the proposed FWSR and also the distribution main to the proposed Development. In order to save energy and minimize the pressure within the water supply network, it is proposed that the

distribution main will directly running down the proposed slopes behind the upper platform to provide fresh water supply to the Development at the upper platform. The distribution main will then pass through the upper platform and run along the proposed public access road to connect to the Development at the lower platform. Since a section of the FWM would need to be laid within the upper platform near the slope toe, a designated Waterworks Reserve (WWR) would be required to facilitate future maintenance of the water mains by WSD. HD has been consulted on the above arrangement of the distribution mains and has expressed no objection to such arrangement.

7.2.4 The preliminary layout of the proposed fresh water system is shown in *Drawing Nos* 199086/BIN/WIA/014 to 017.

#### 7.3 Preliminary Design - Flushing Water

**Inlet of the Proposed Pumping Station** 

Interim scenario

7.3.1 DN200 SWM is proposed to be branched off from the existing DN1000 SWM output from the existing TKT SWSR and to be connected to the inlet of the proposed pumping station.

Ultimate scenario

7.3.2 DN200 flushing water main is proposed to be branched off from the planned reclaimed water main output from the planned TKT RWSR under YLS Development and HSK NDA and to be connected to the inlet of the proposed pumping station. The exact connection point is to be confirmed with the project teams of YLS Development/ HSK NDA.

Outlet of the Proposed Pumping Station and Inlet of the Proposed FLWSR

7.3.3 DN200 flushing water main/ DN200 SWM is proposed as the outlet of the proposed pumping station and to be laid along existing and proposed WSD's maintenance access to reach the proposed FLWSR.

Outlet of the Proposed FLWSR

7.3.4 DN200 flushing water main/ DN200 SWM is proposed as the outlet of the proposed flushing water service reservoir as well as the distribution main to the Development. In order to save energy and minimize the pressure within the water supply network, the DN200 flushing water main/ DN200 SWM is proposed to be directly running down the proposed slopes behind the upper platform to provide flushing water supply to the Development at the upper platform. The DN200 flushing water main/ DN200 SWM is then proposed to be passing through the upper platform and running along the proposed public access road to connect to the Development at the lower platform. Since a section of the SWM would need to be laid within the upper platform near the slope toe, a designated Waterworks Reserve (WWR) would be required to facilitate future maintenance of the water mains by WSD. The location of the WWR can be referred to *Drawings Nos.* 199086/BIN/WIA/016 and 022.



- 7.3.5 The proposed interim flushing water supply system is shown in *Drawing Nos.* 199086/BIN/WIA/018 to 020.
- 7.3.6 The preliminary layout of the proposed ultimate flushing water mains is shown in *Drawing Nos.* 199086/BIN/WIA/021 to 023.

#### 7.4 Cost Estimation and Programme of Works

7.4.1 Detailed cost estimation and programme of works are discussed in the "Implementation Programme, Cash Flow Schedule, Recurrent Consequence and Cost Estimate" respectively which were submitted under separate cover.

#### 8 CONCLUSION AND RECOMMENDATION

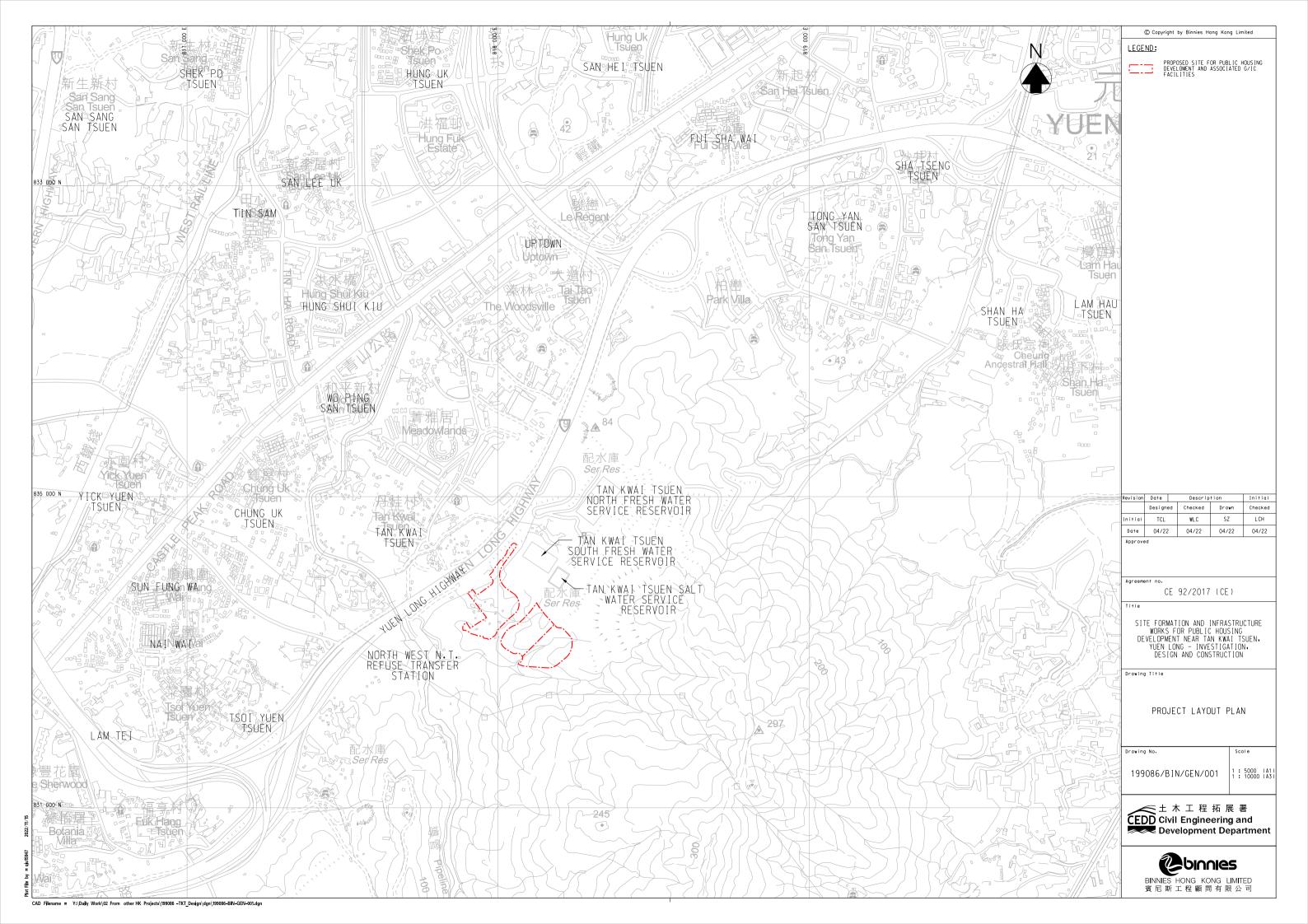
- 8.1.1 An impact assessment on the existing and planned water supplies system due to the additional water demand arising from the Development was conducted.
- 8.1.2 Based on the latest development parameters, it is estimated that the fresh and flushing water demands due to the Development are 5,554 and 1,508 cubic meters per day in total respectively.
- 8.1.3 According to the as-built records, there would be insufficient residual head for both fresh water and flushing water supplies for the Development with high level building platforms by the existing water supply facilities. In addition, there would also be a shortfall in capacity of LOP SWPS for salt water supply for the Development.
- 8.1.4 The design capacity of LOP SWPS is 83MLD for delivering SW to areas including Tin Shui Wai and Yuen Long Town. When the supply of reclaimed water to be available later, we understand that WSD will switch the FLW supply from salt water to reclaimed water for areas including Tin Shui Wai and Yuen Long Town so that the loading on LOP SWPS will be reduced significantly. In this connection, upgrading of LOP SWPS for the proposed development may only be required if the progress of reclaimed water supply for Tin Shui Wai and Yuen Long Town would be delayed. Fresh water augmentation for flushing will be adopted as the last resort in emergency situation when the supply from salt water is not available.
- 8.1.5 A high-level fresh water service reservoir with design capacity of approximately 8,100m³, a high-level flushing water service reservoir with design capacity of approximately 1,000m³ and an associated pumping station have been proposed to provide sufficient residual head and reliable water supplies for the proposed Development site. Access roads will be provided to facilitate the operation and maintenance of the new water supply facilities.
- 8.1.6 It is anticipated that diversion/relocation/removal of the existing mains due to the proposed site formation and infrastructure works under the project will be needed. Details of the diversion or relocation scheme will be proposed in the later stage of the project.
- 8.1.7 The proposed pumping station is branched off from the existing DN1000 FWM and connect to the proposed DN300 FWM. Fresh water will be pumped to the proposed fresh water service reservoir and connect to proposed DN450 FWM to provide fresh water supplies for the Development. Also, distribution mains of size DN300 supplying fresh water is proposed to be laid with fire hydrants staggered every 300m along alternate sides of the proposed access road to be connected to Tin Shui Wai West Interchange (TSWWI) slip road and Shun Tat Road, as well as the access road between housing platforms. Hydraulic analysis has been carried out and the proposal is found to be technically feasible.
- 8.1.8 The proposed pumping station is branched off from the existing DN1000 flushing water main and connect to the proposed DN200 flushing water main. Flushing water will be pumped to the proposed flushing water service reservoir and connect to proposed DN200 flushing water main to provide flushing water supplies for the

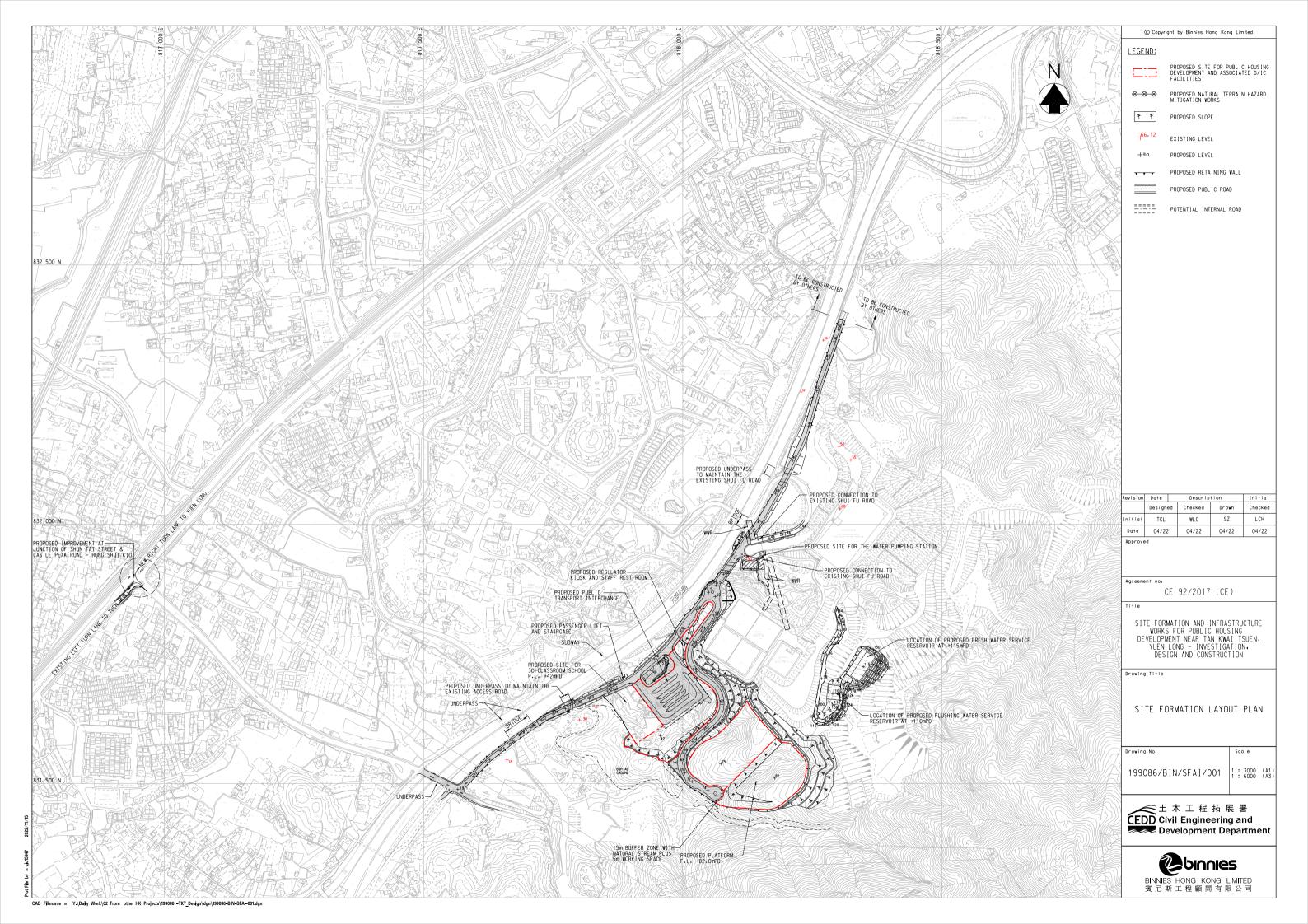
Agreement No. CE 92/2017 (CE)
Site Formation and Infrastructure Works for Public Housing
Development near Tan Kwai Tsuen, Yuen Long
- Investigation, Design and Construction

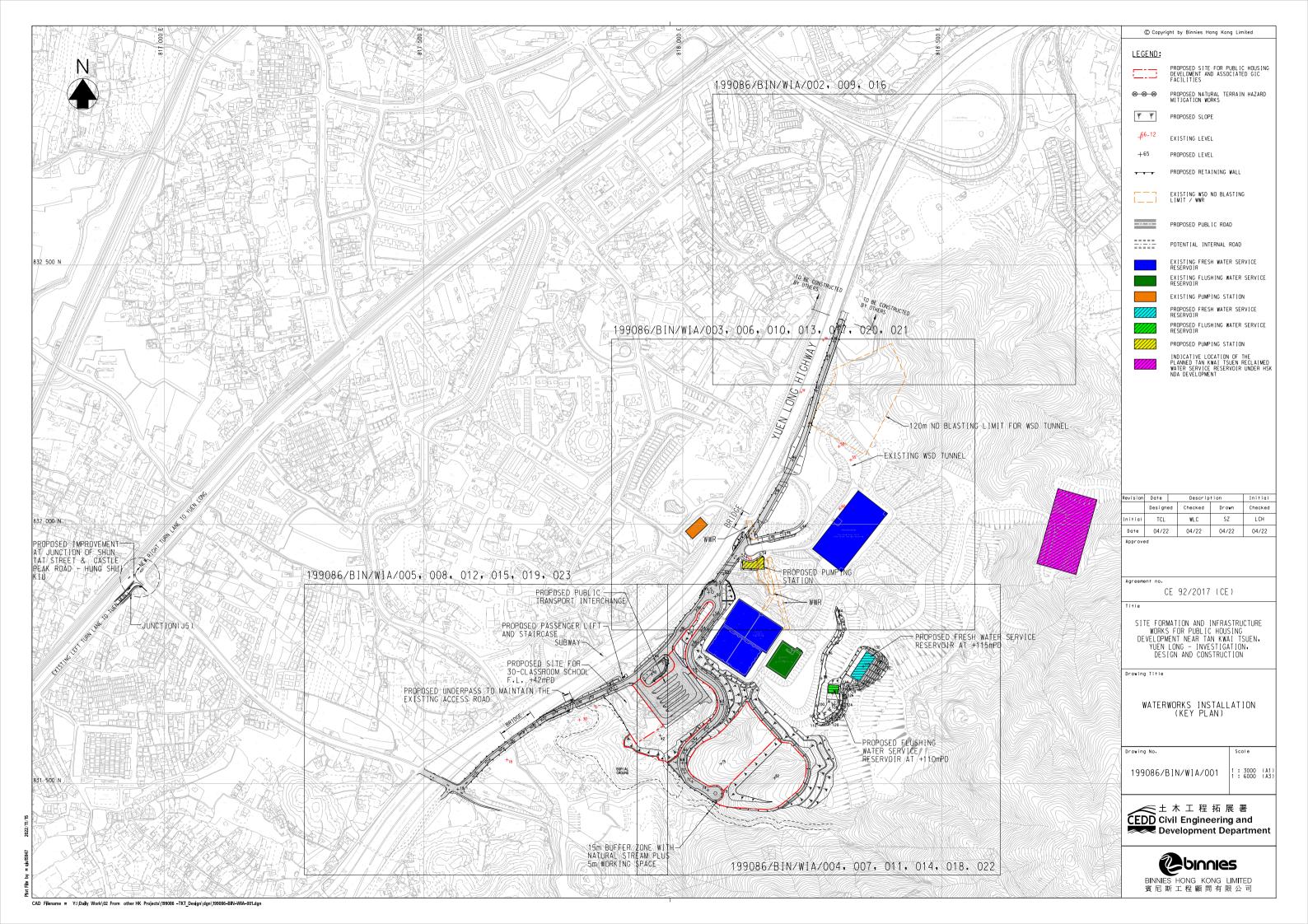
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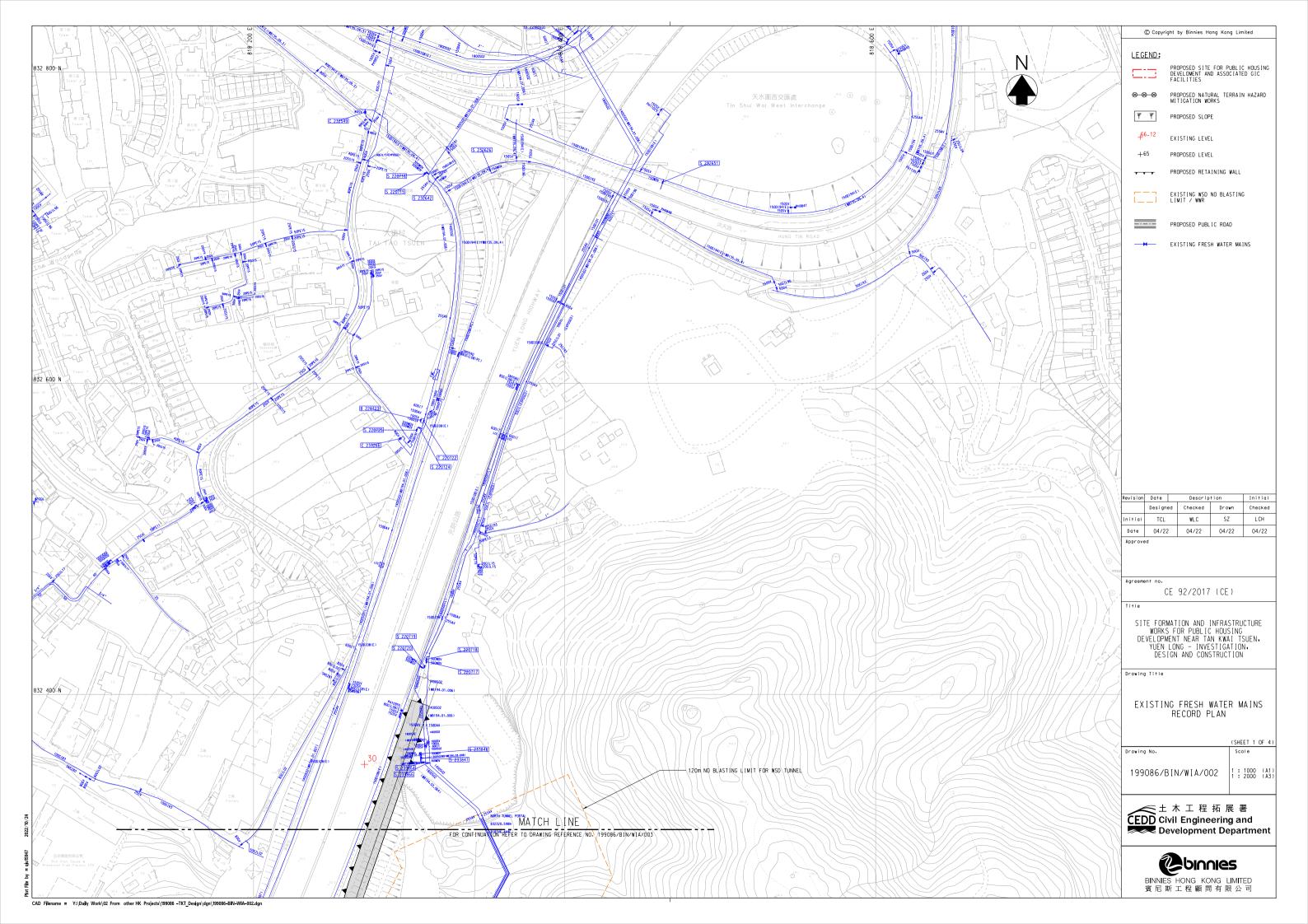
**FIGURES** 

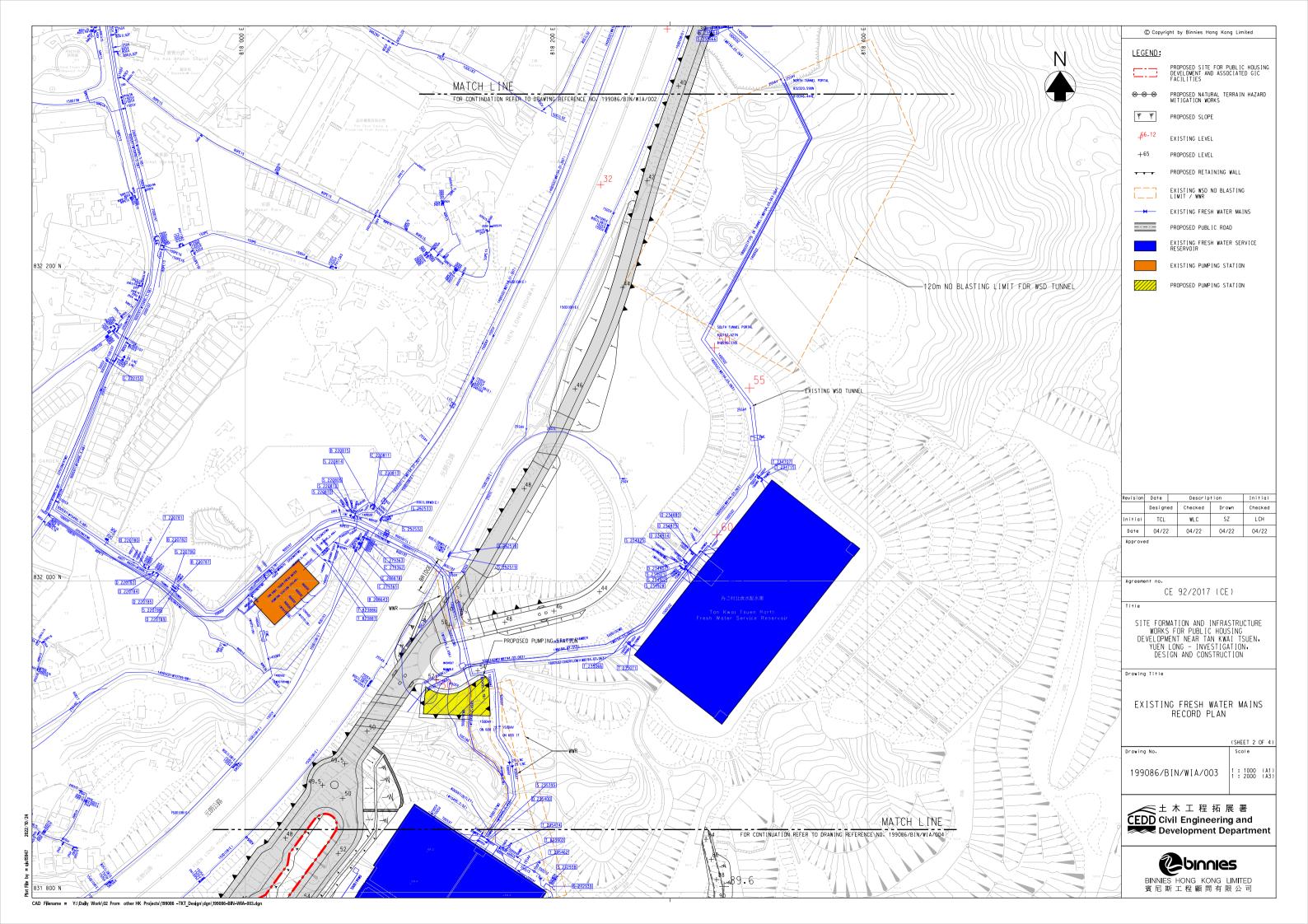


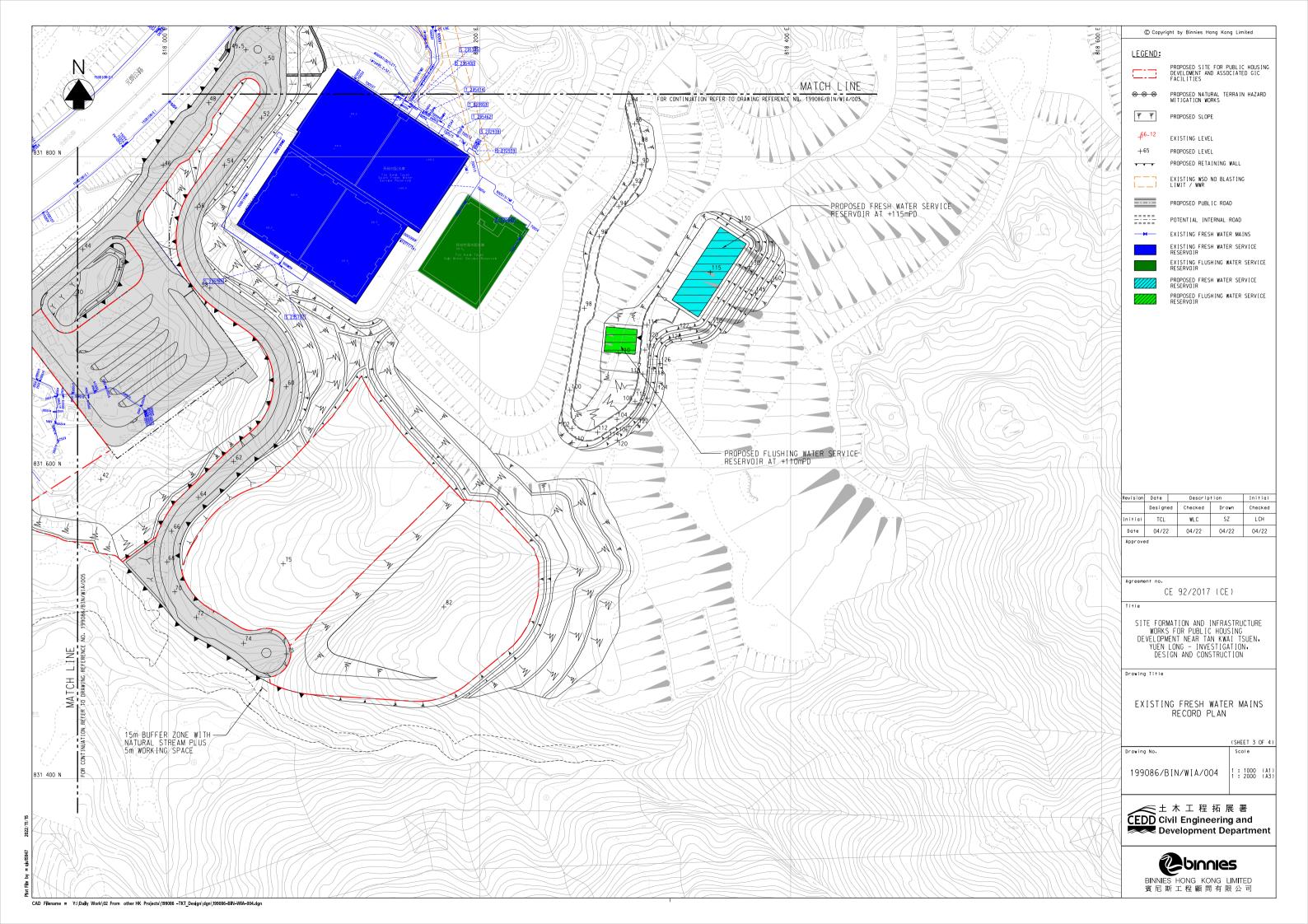


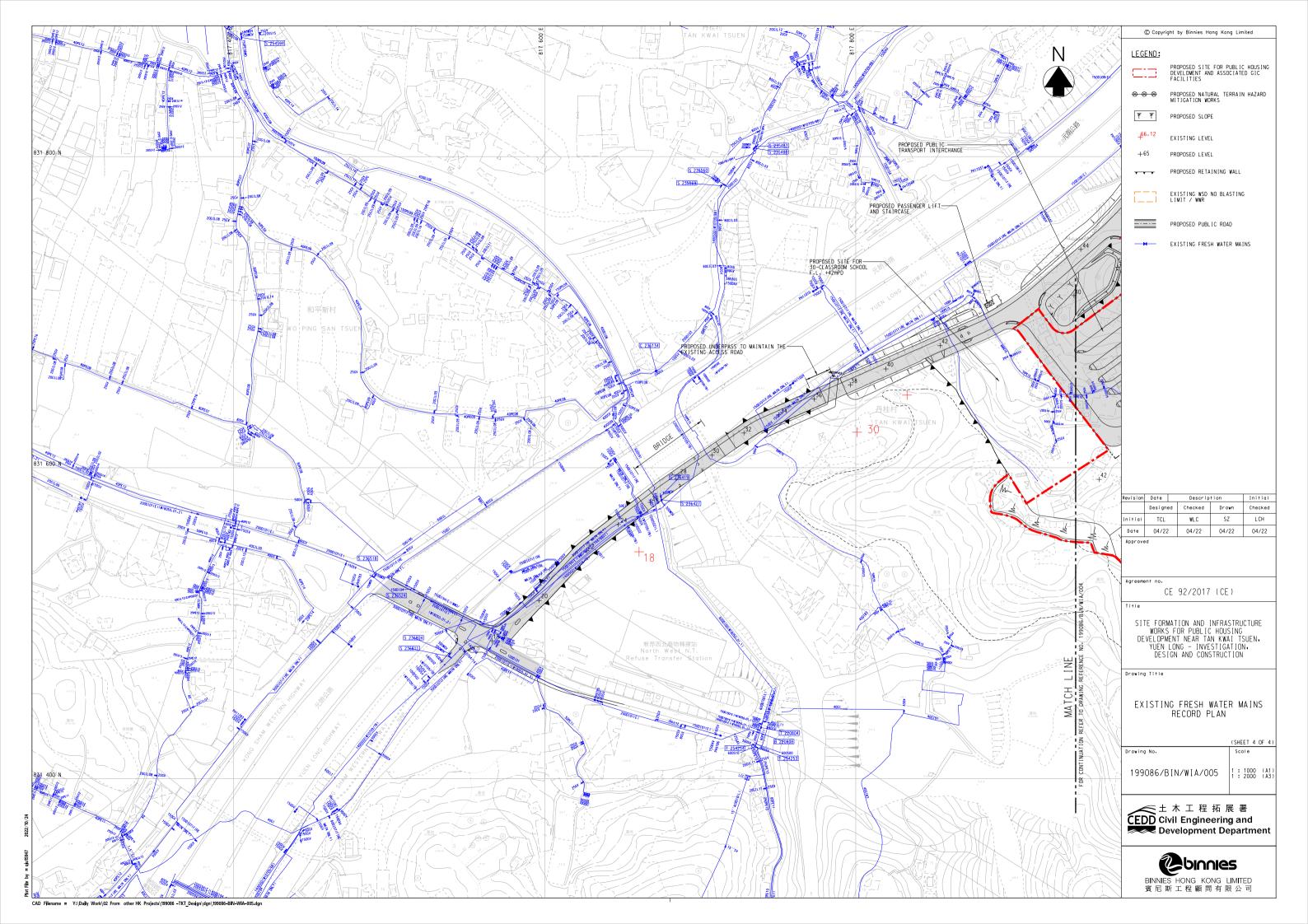


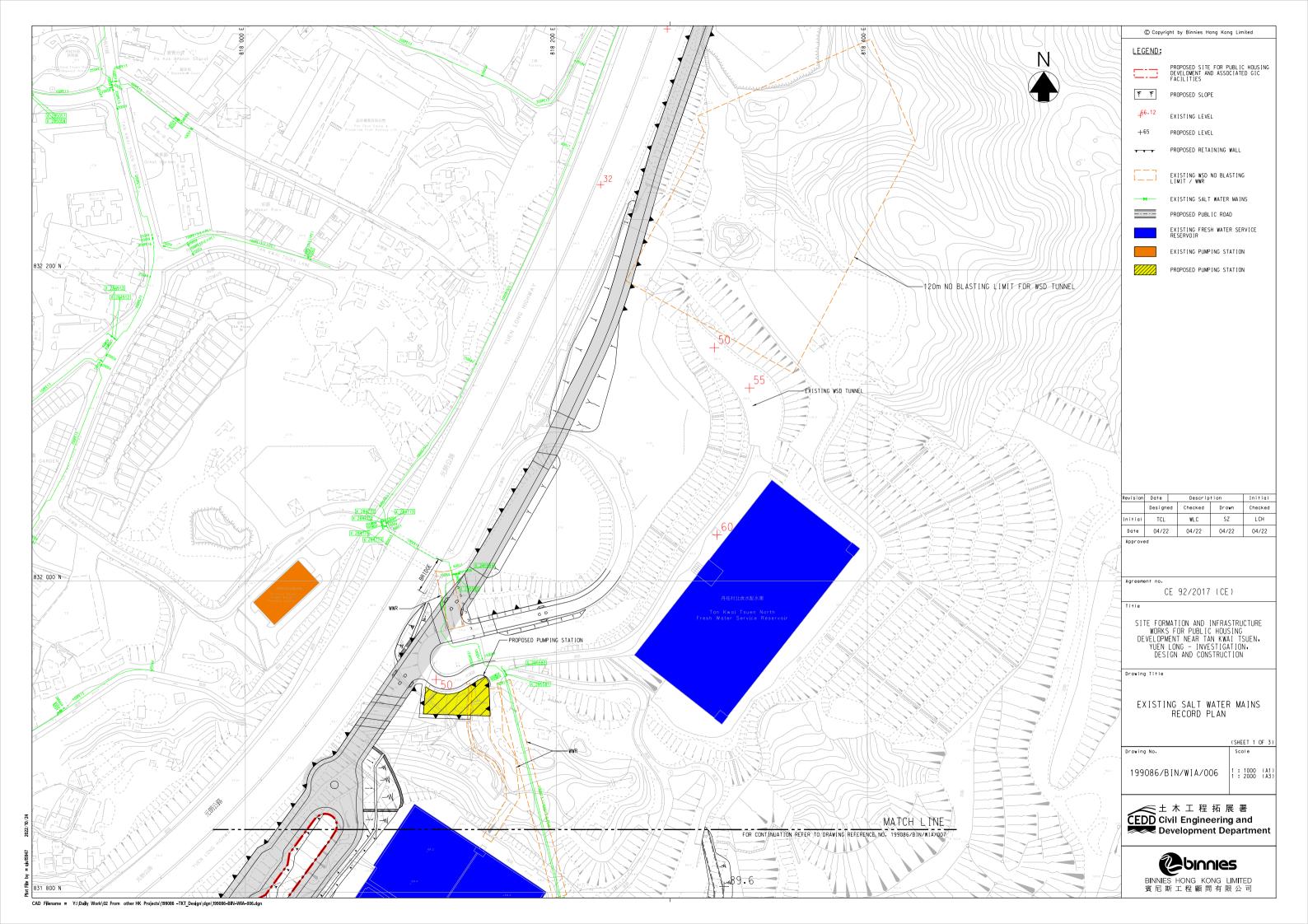


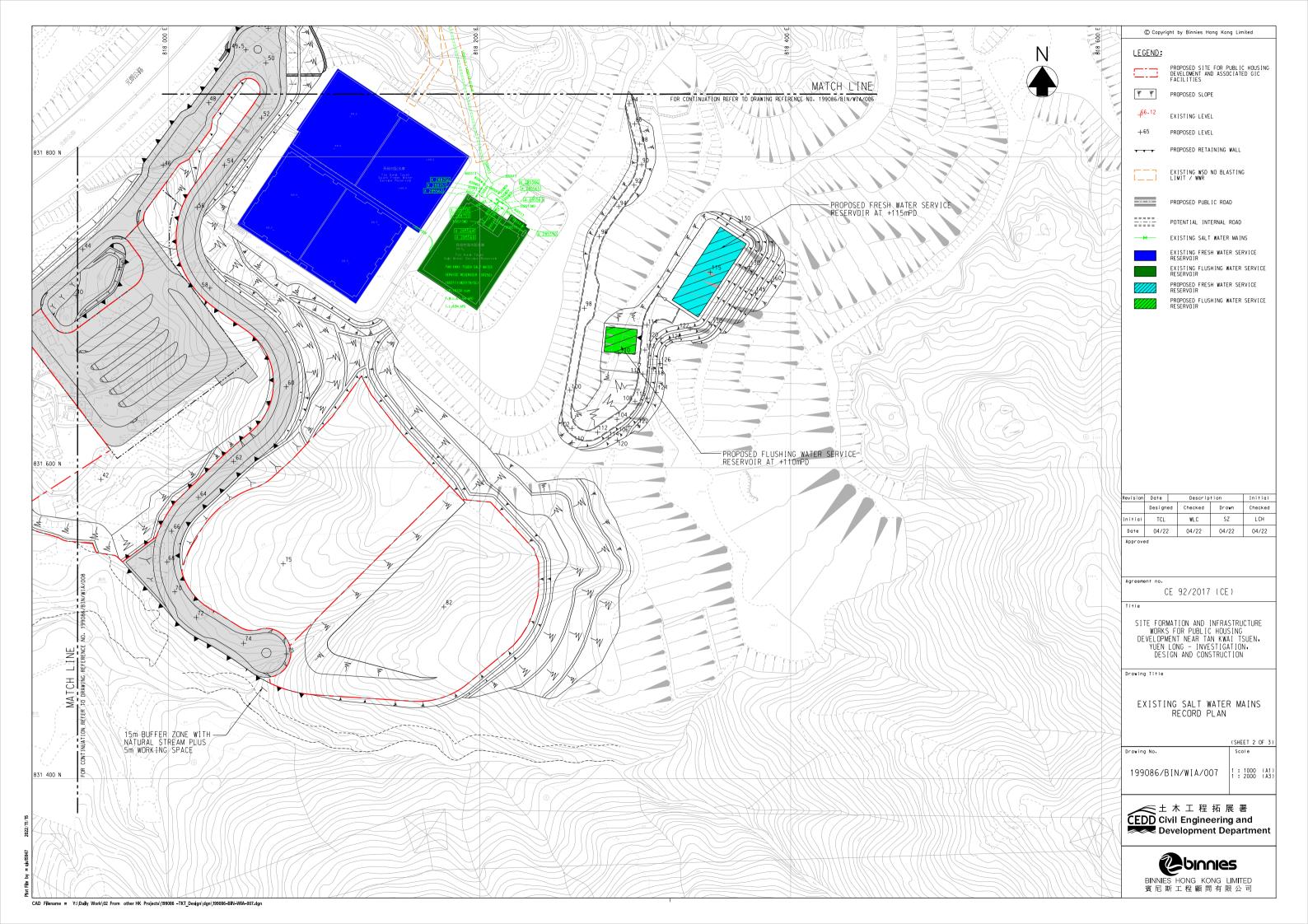


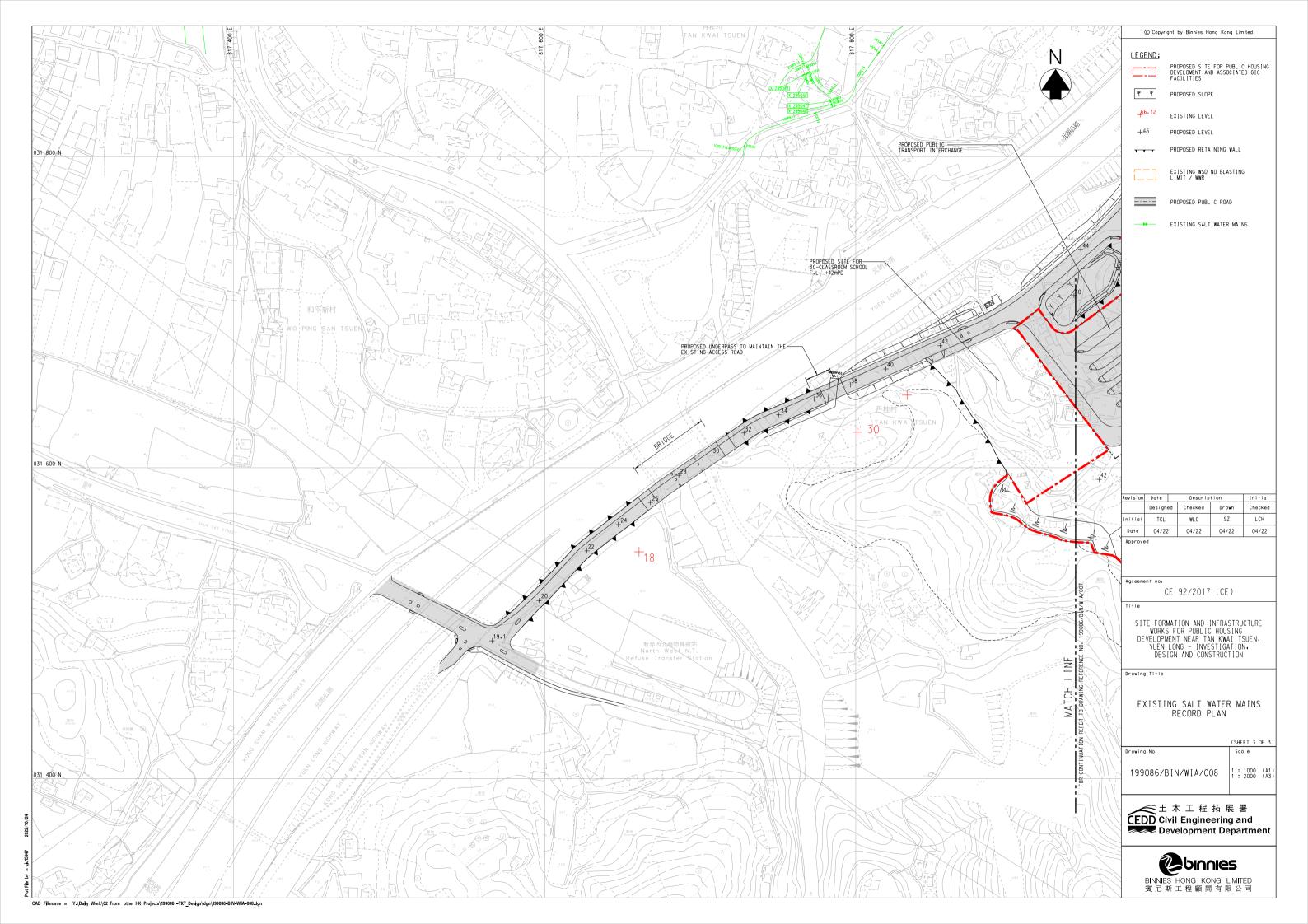


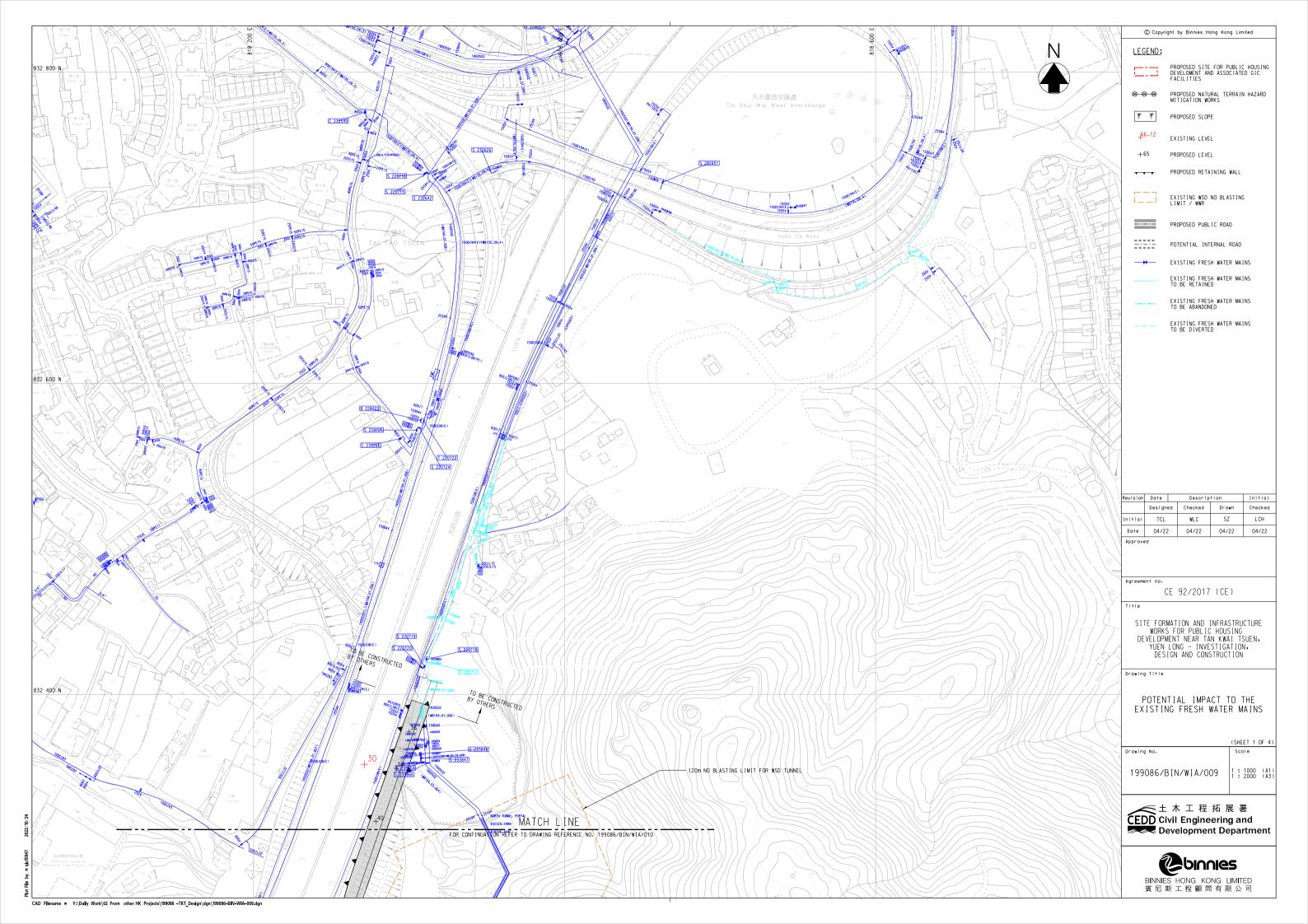


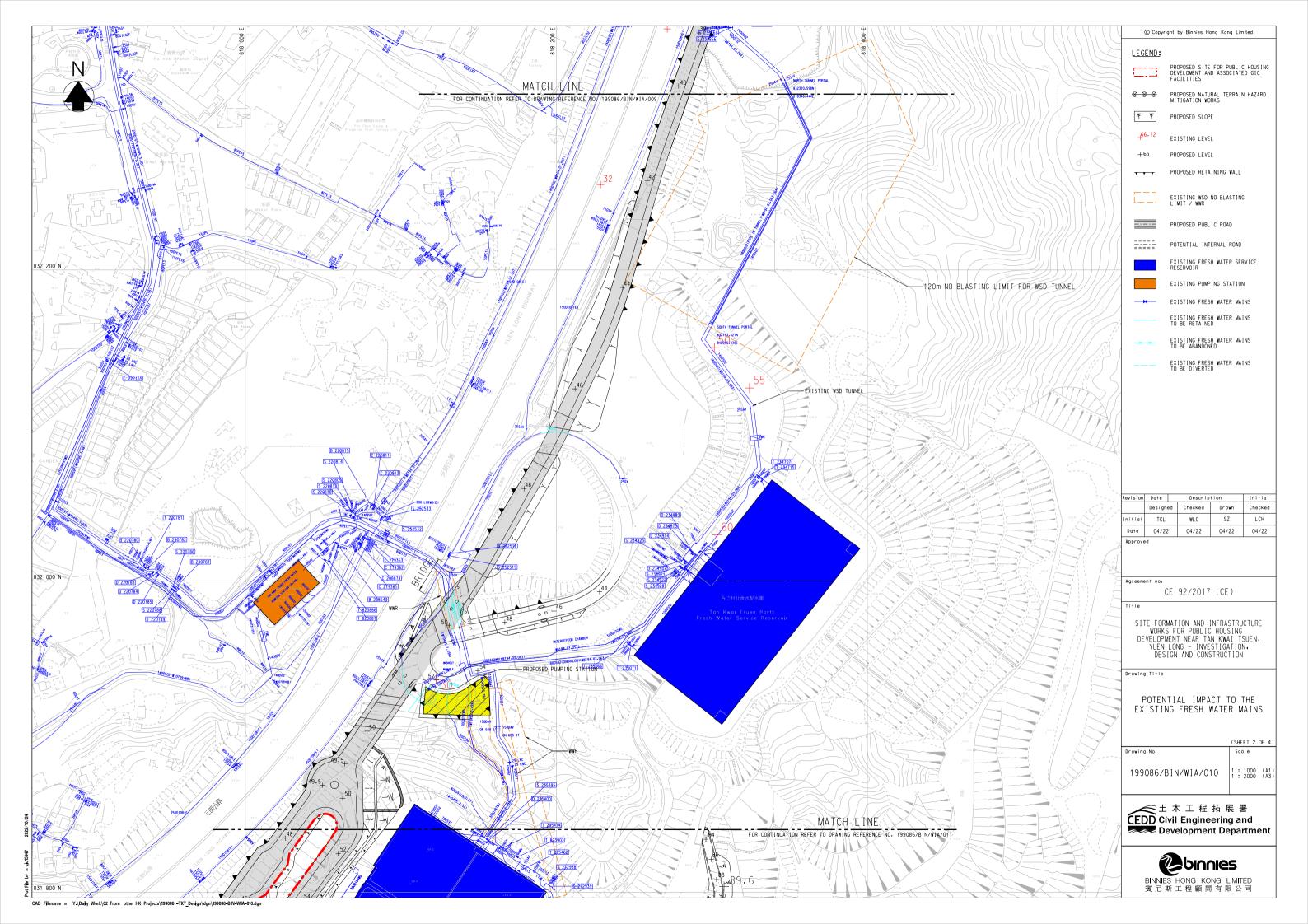


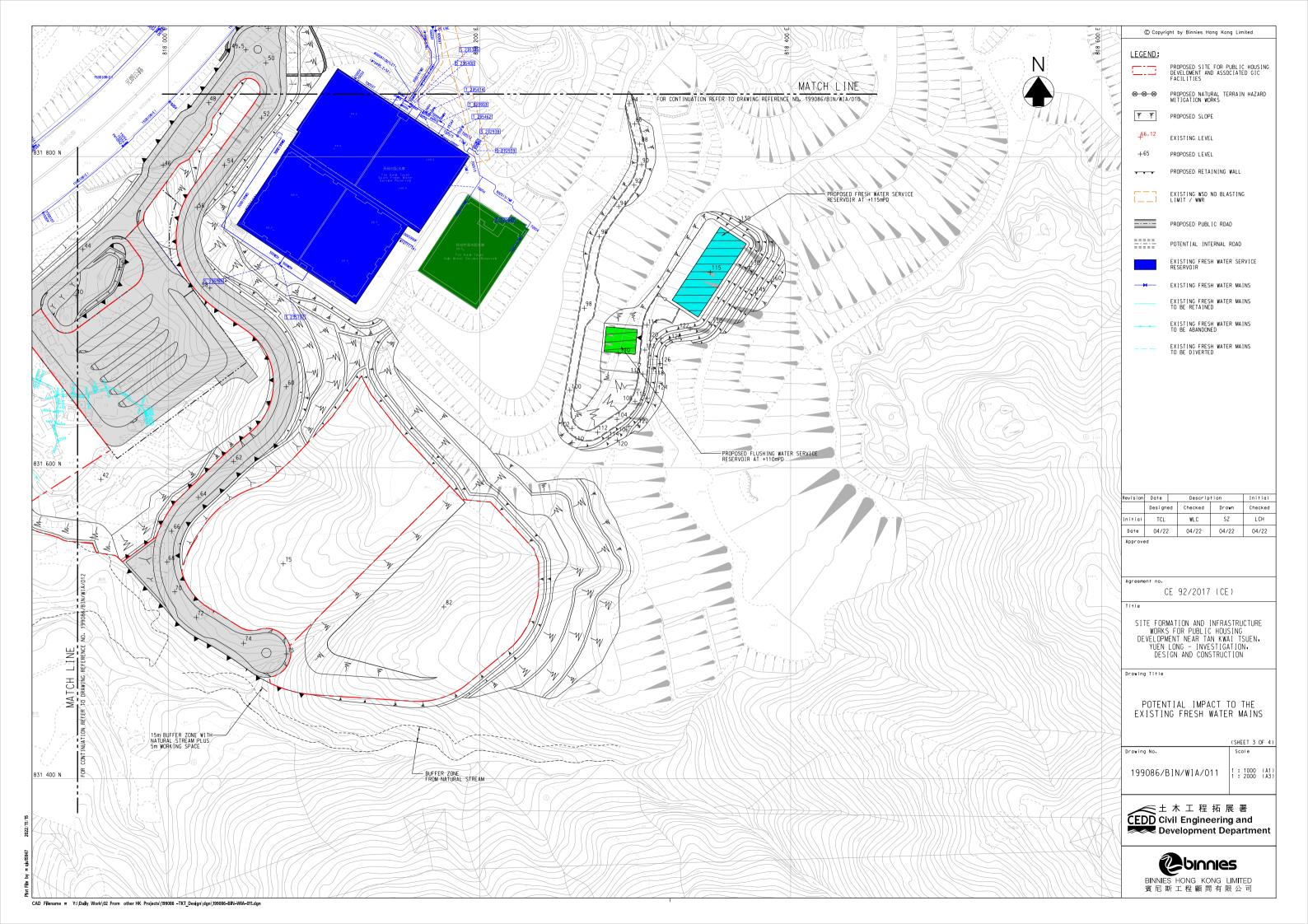


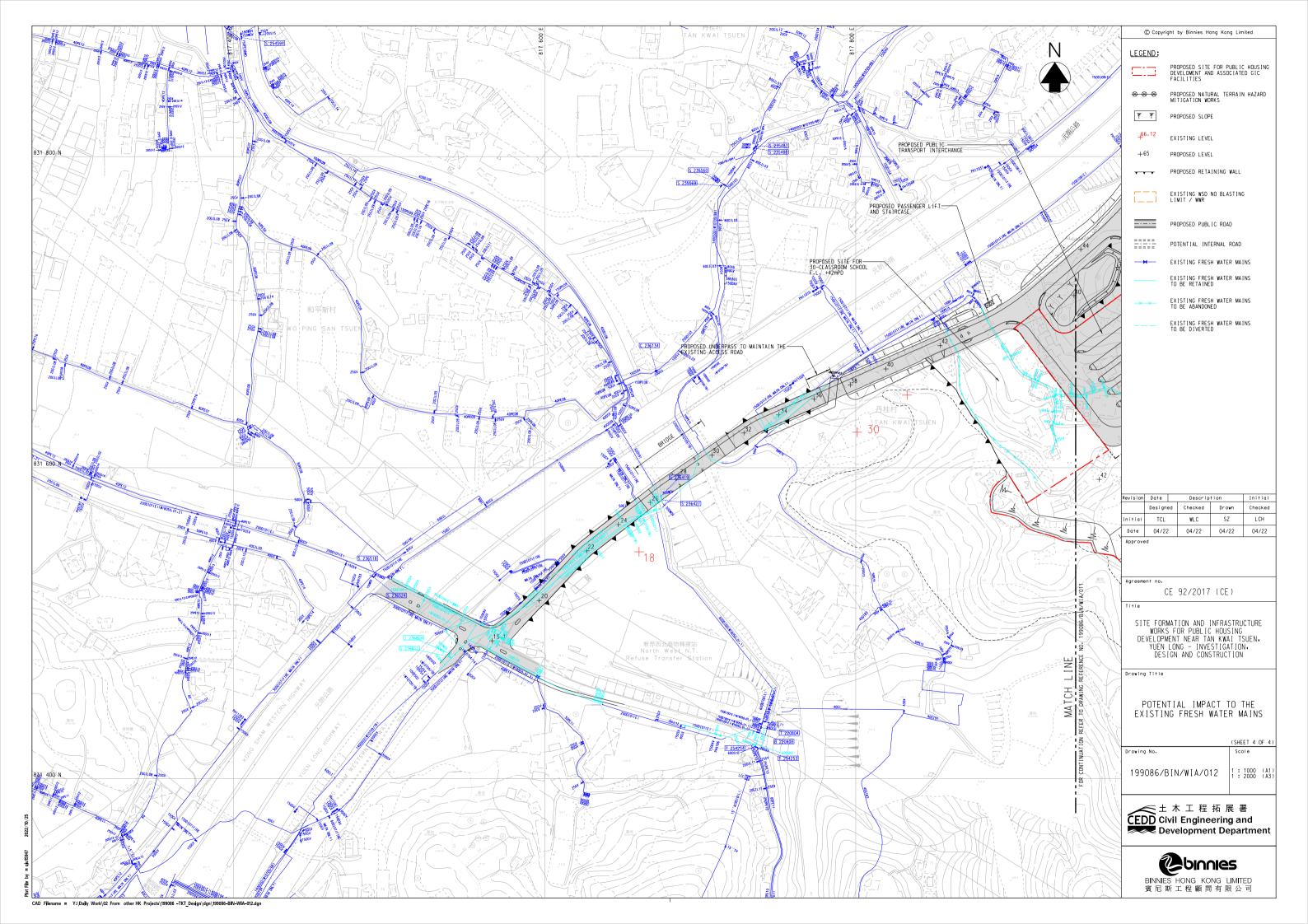


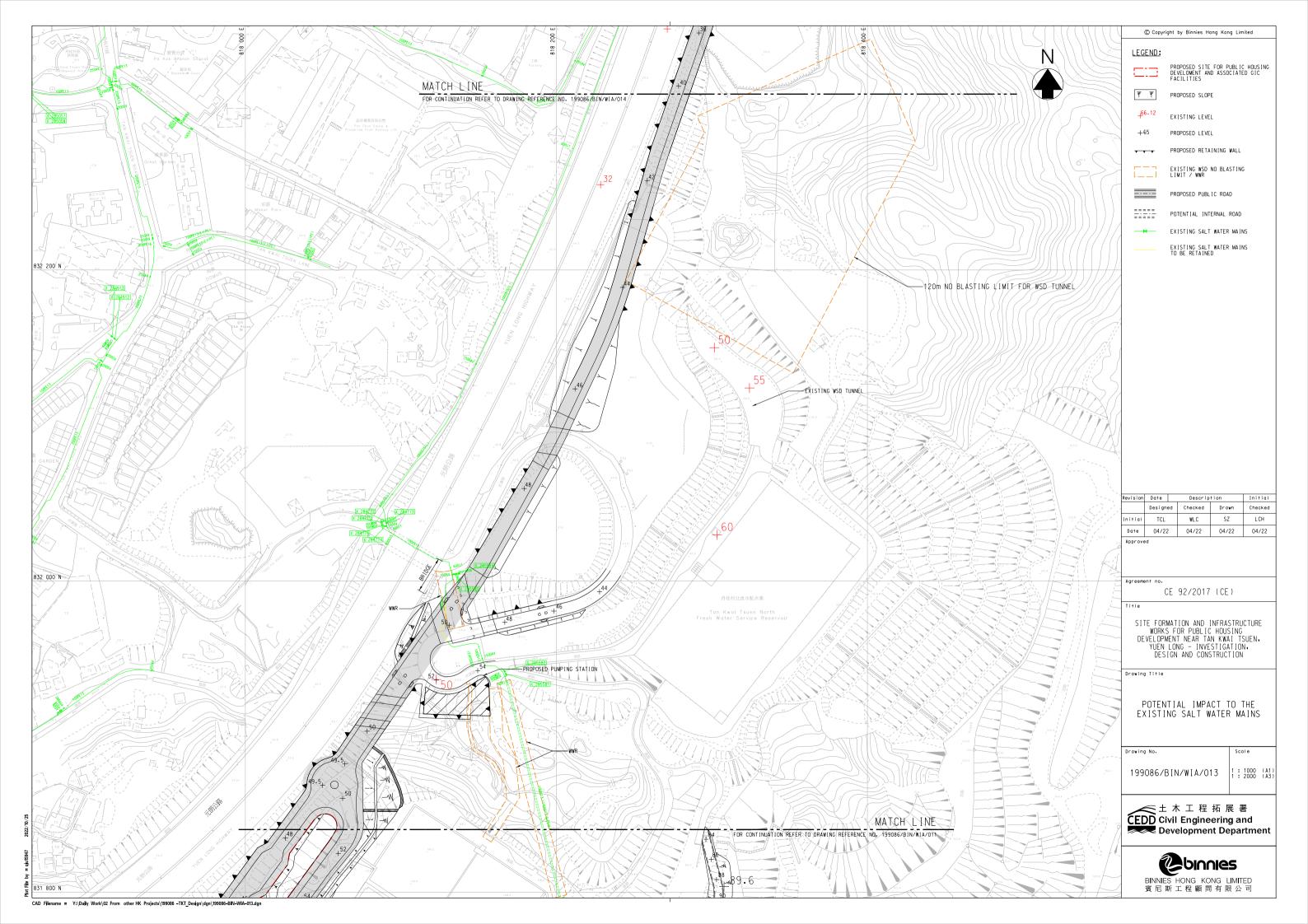




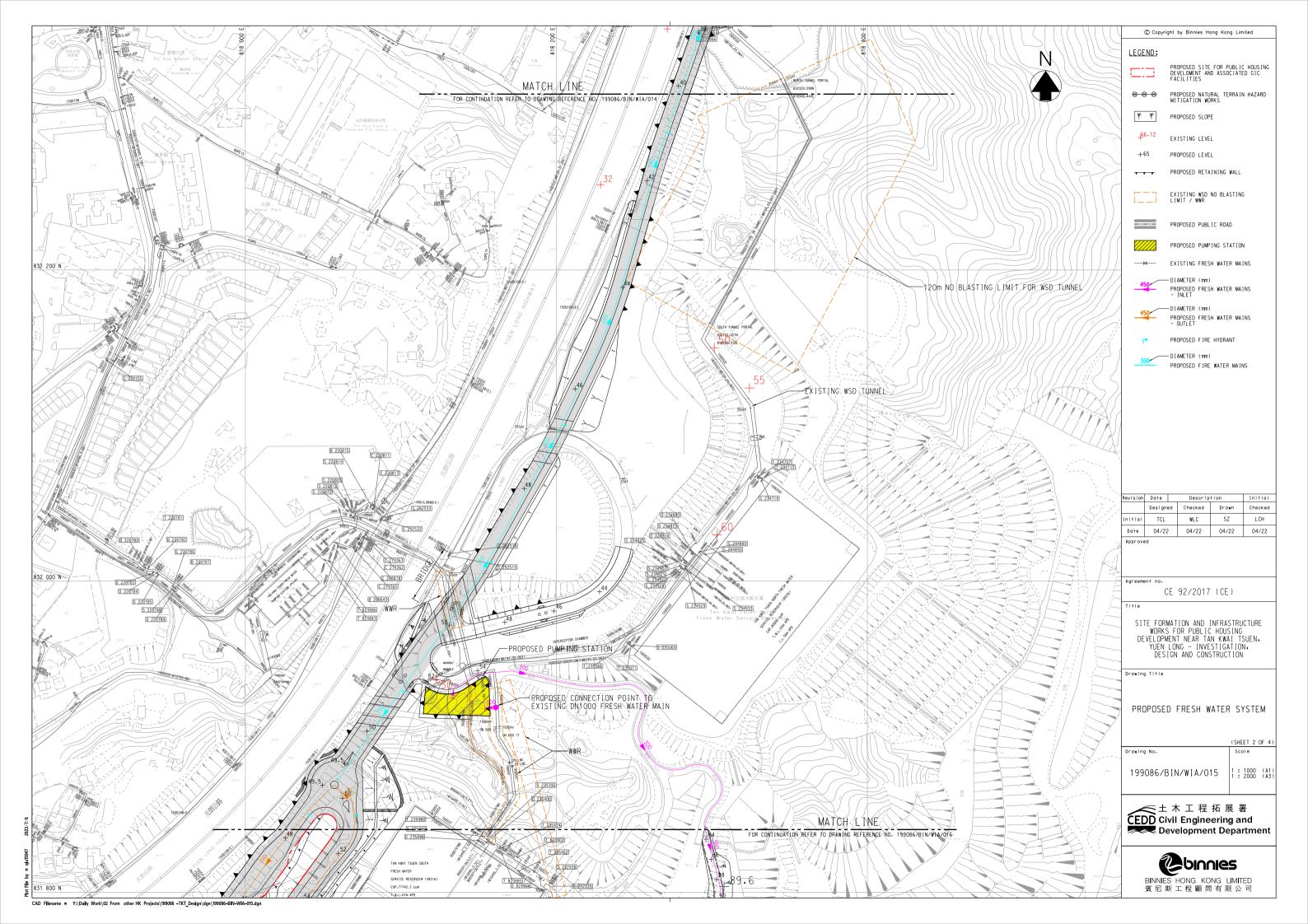


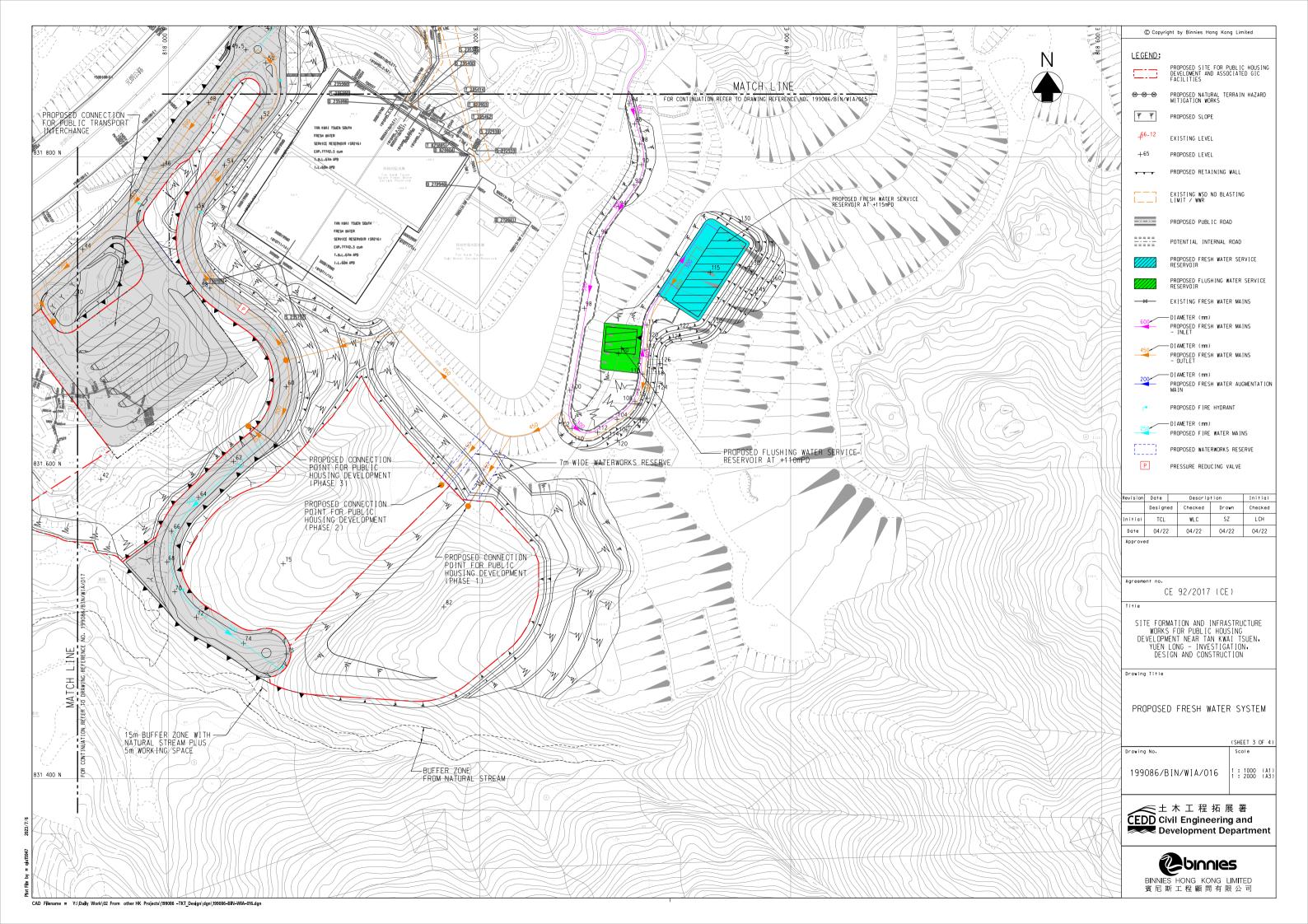


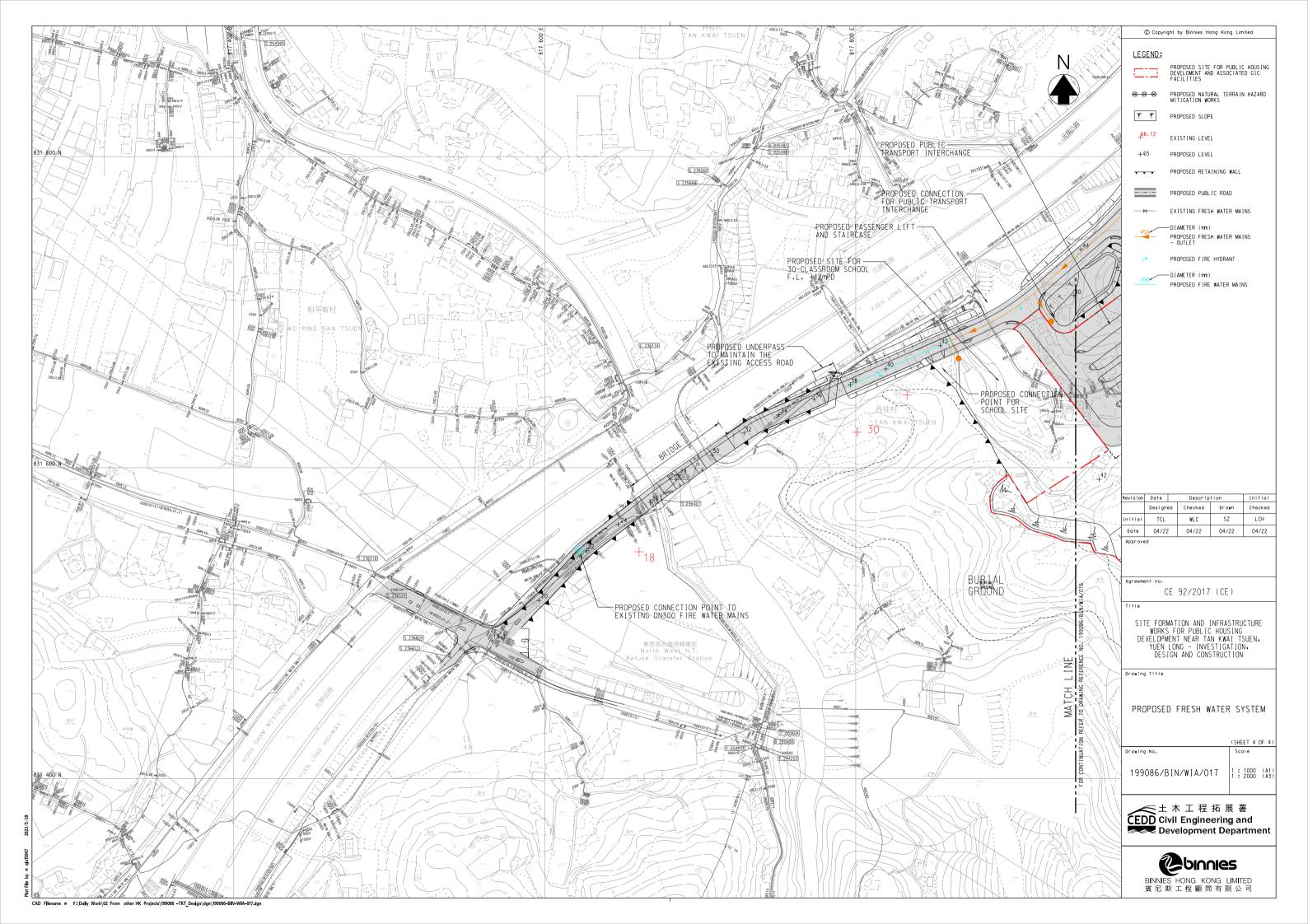


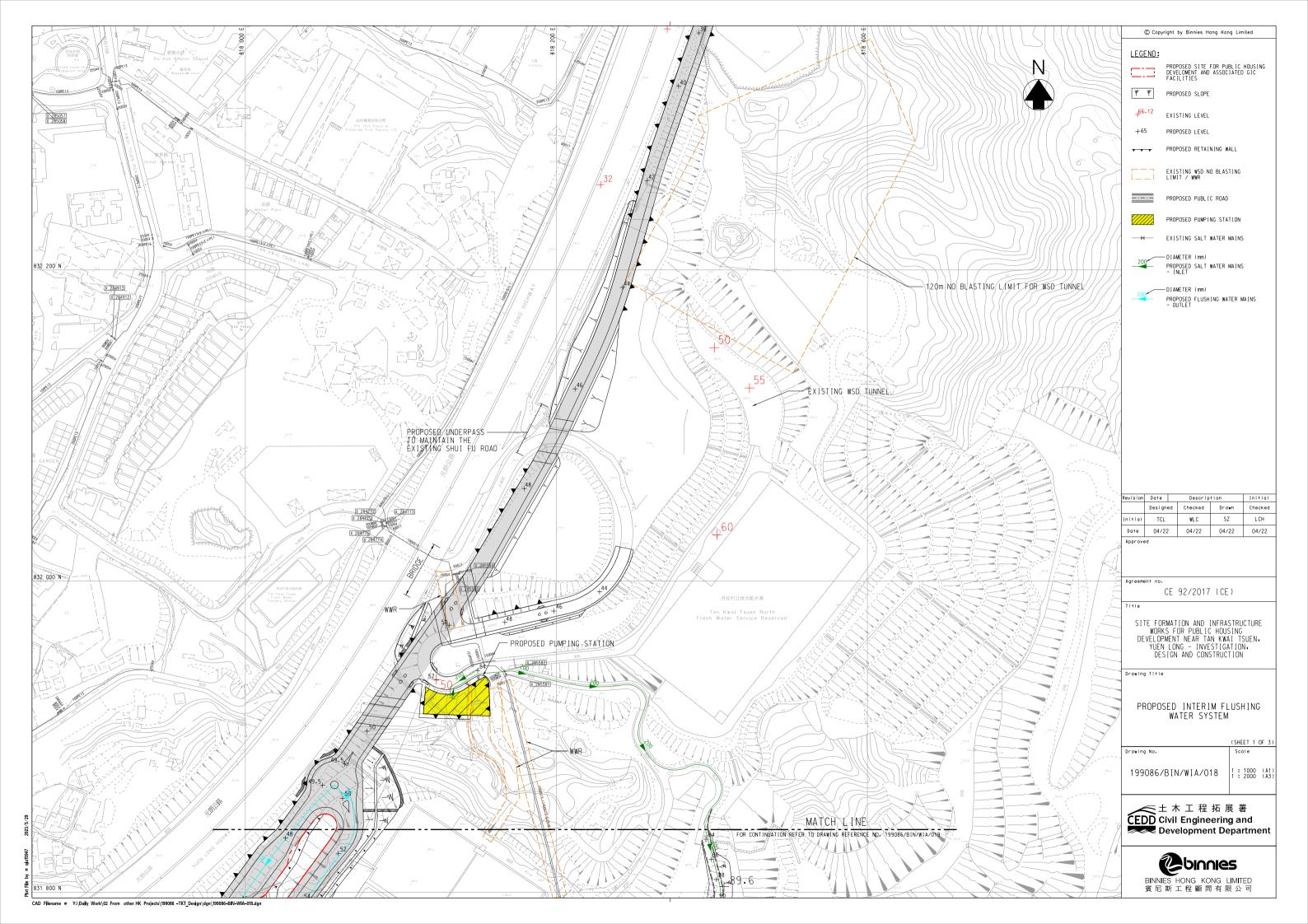


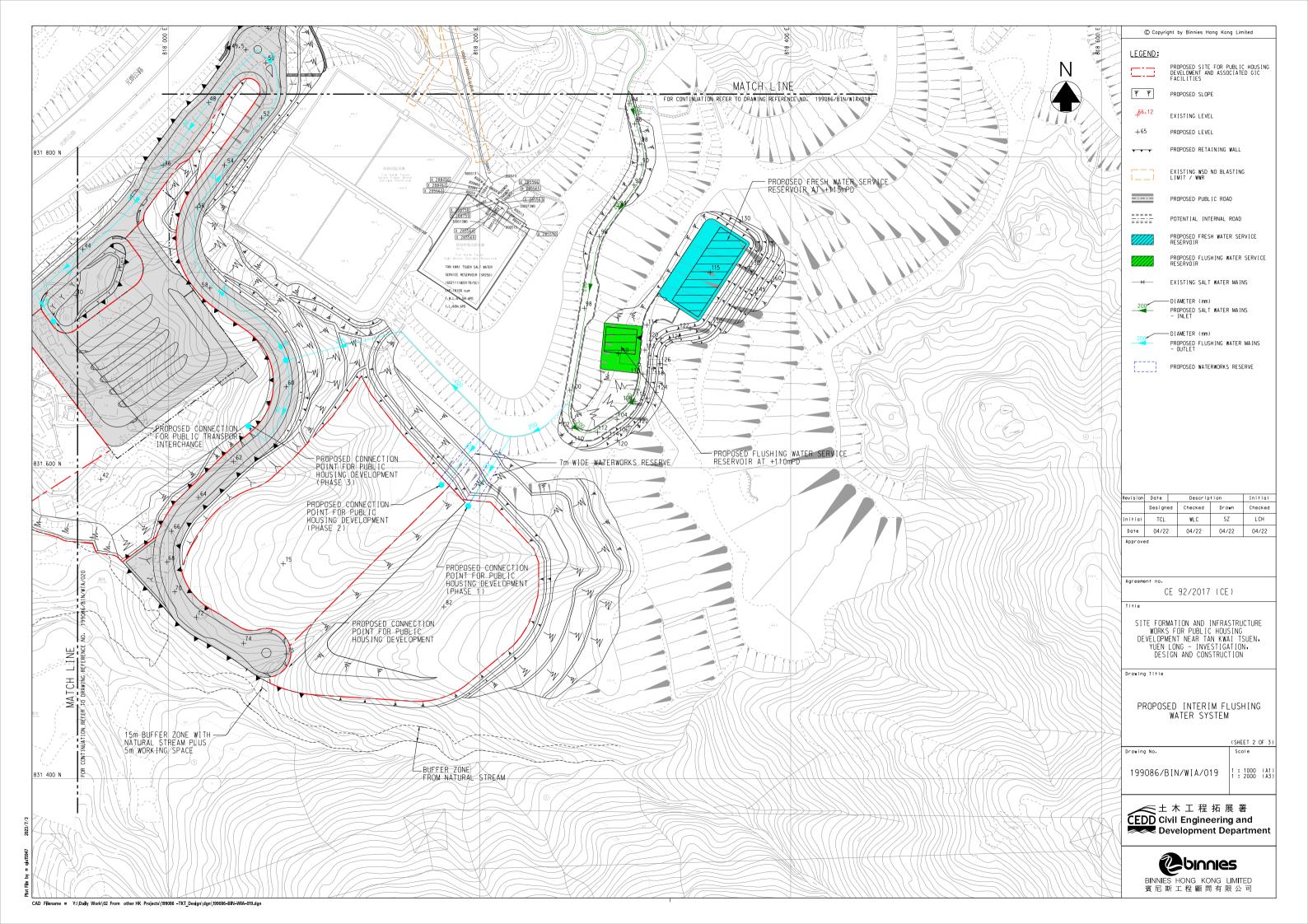


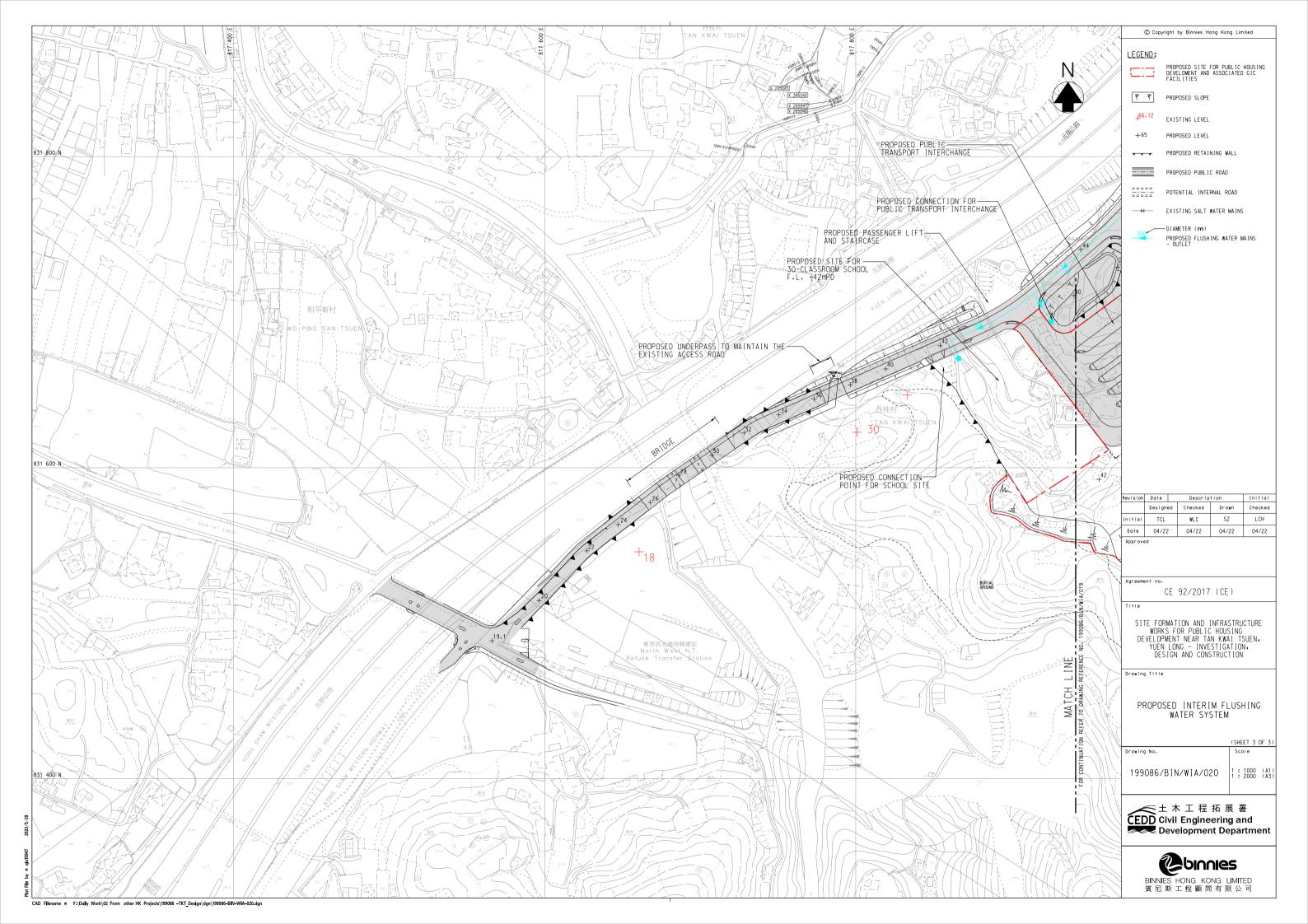


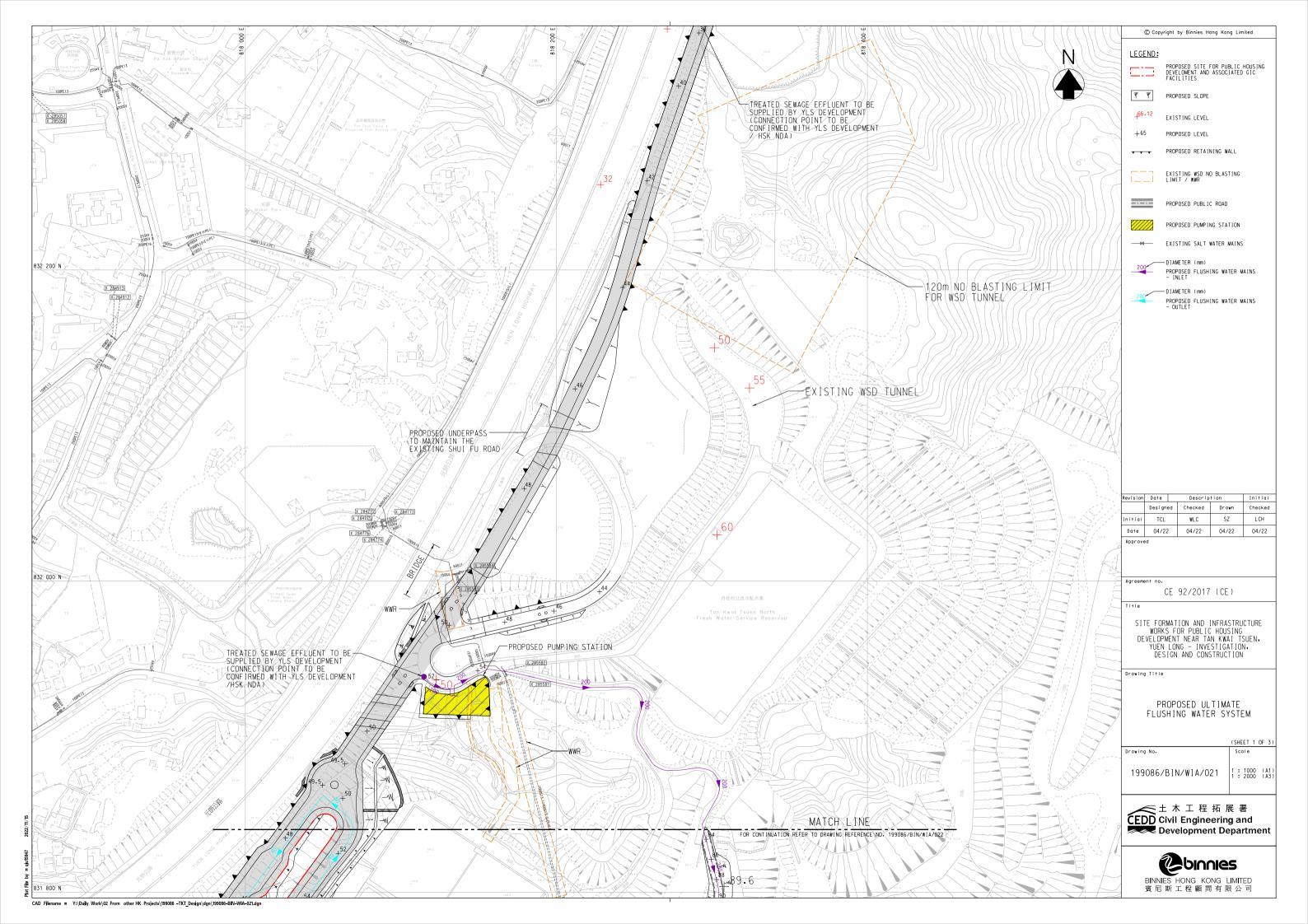


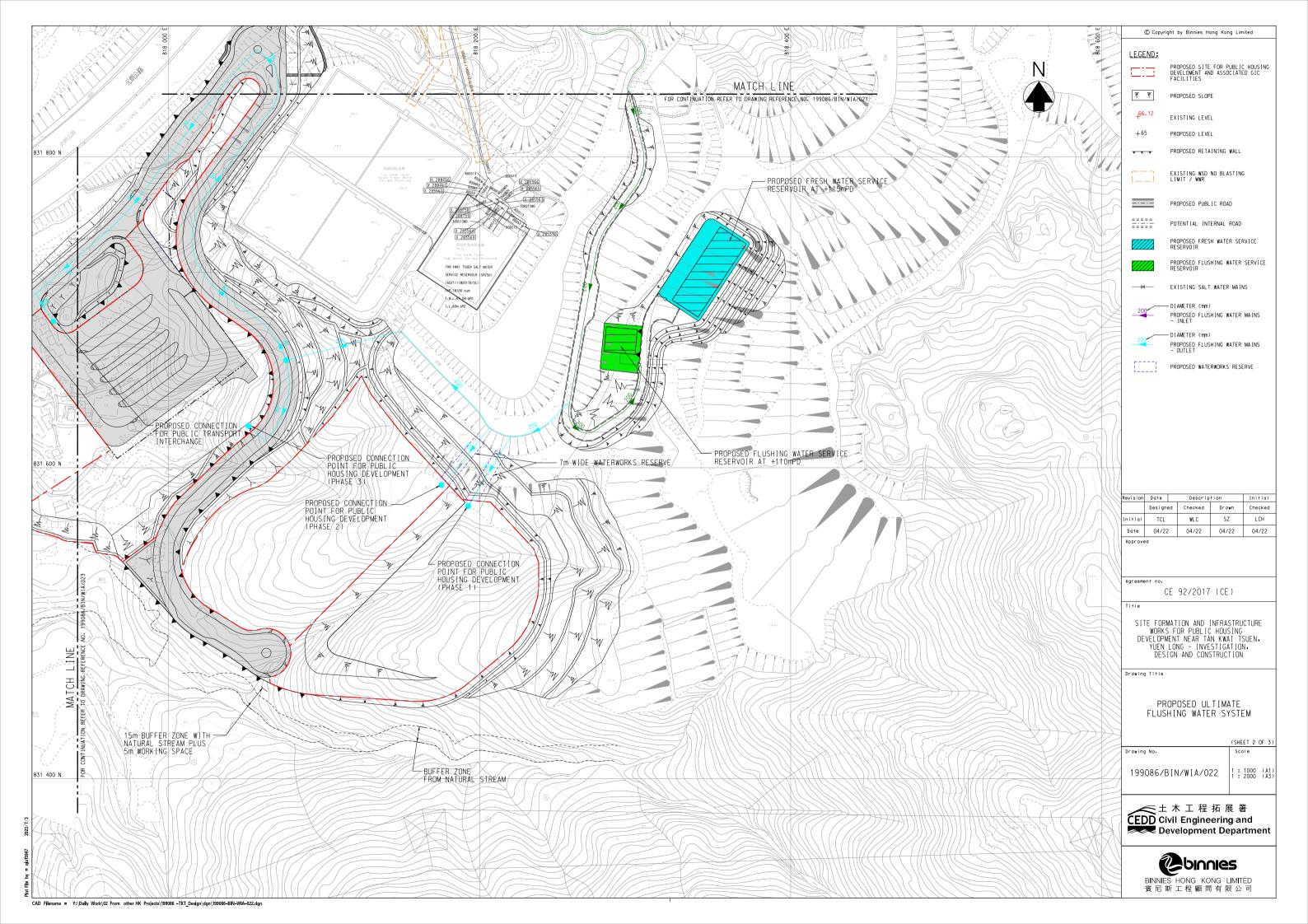


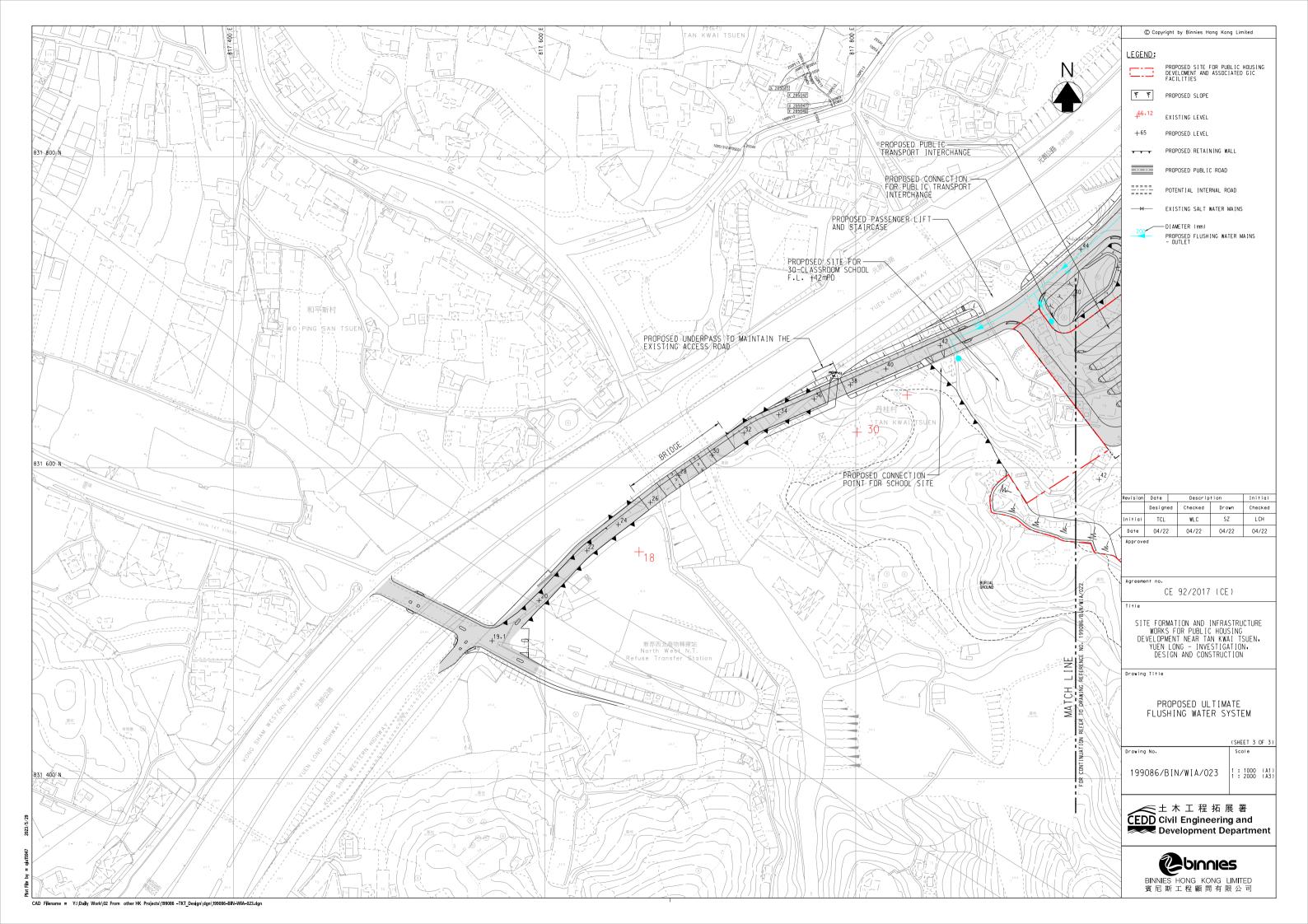


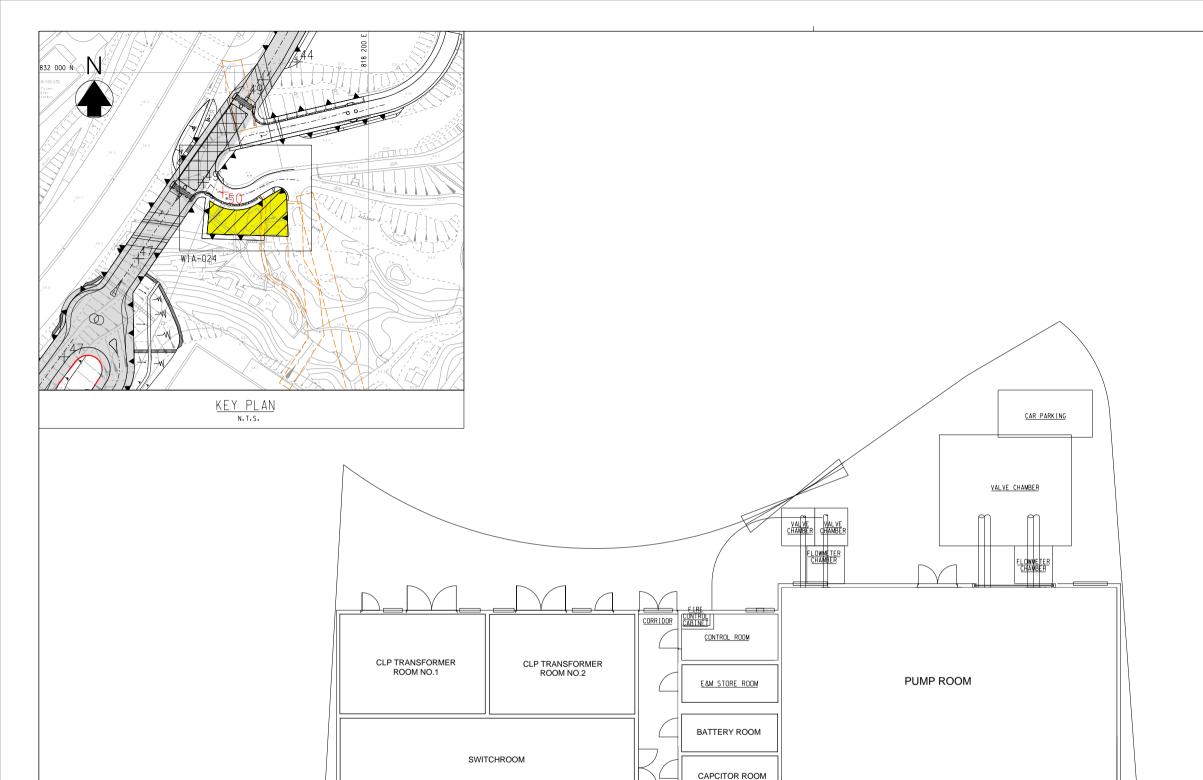












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### NOTES:

1. ALL DIMENSIONS ARE IN MILLIMETRES.

2. ALL LEVELS ARE IN METRES ABOVE PRINCIPAL DATUM.

LEGEND:

DOUBLE-LEAF DOOR

CABLE / PIPE TRENCH WITH CHEQUER PLATE OR GOATING COVER

 Revision
 Date
 Description
 Initial

 Designed
 Checked
 Drawn
 Checked

 Initial
 TCL
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 SZ
 LCH

 Date
 04/22
 04/22
 04/22
 04/22

Annroved

Agreement no.

CE 92/2017 (CE)

itle

SITE FORMATION AND INFRASTRUCTURE WORKS FOR PUBLIC HOUSING DEVELOPMENT NEAR TAN KWAI TSUEN, YUEN LONG - INVESTIGATION, DESIGN AND CONSTRUCTION

Drawing Title

GENERAL ARRANGEMENT OF THE PROPOSED PUMPING STATION

199086/RIN/WIA/0

199086/BIN/WIA/024

土木工程拓展署 CEDD Civil Engineering and Development Department

1 : 100 (A1) 1 : 200 (A3)



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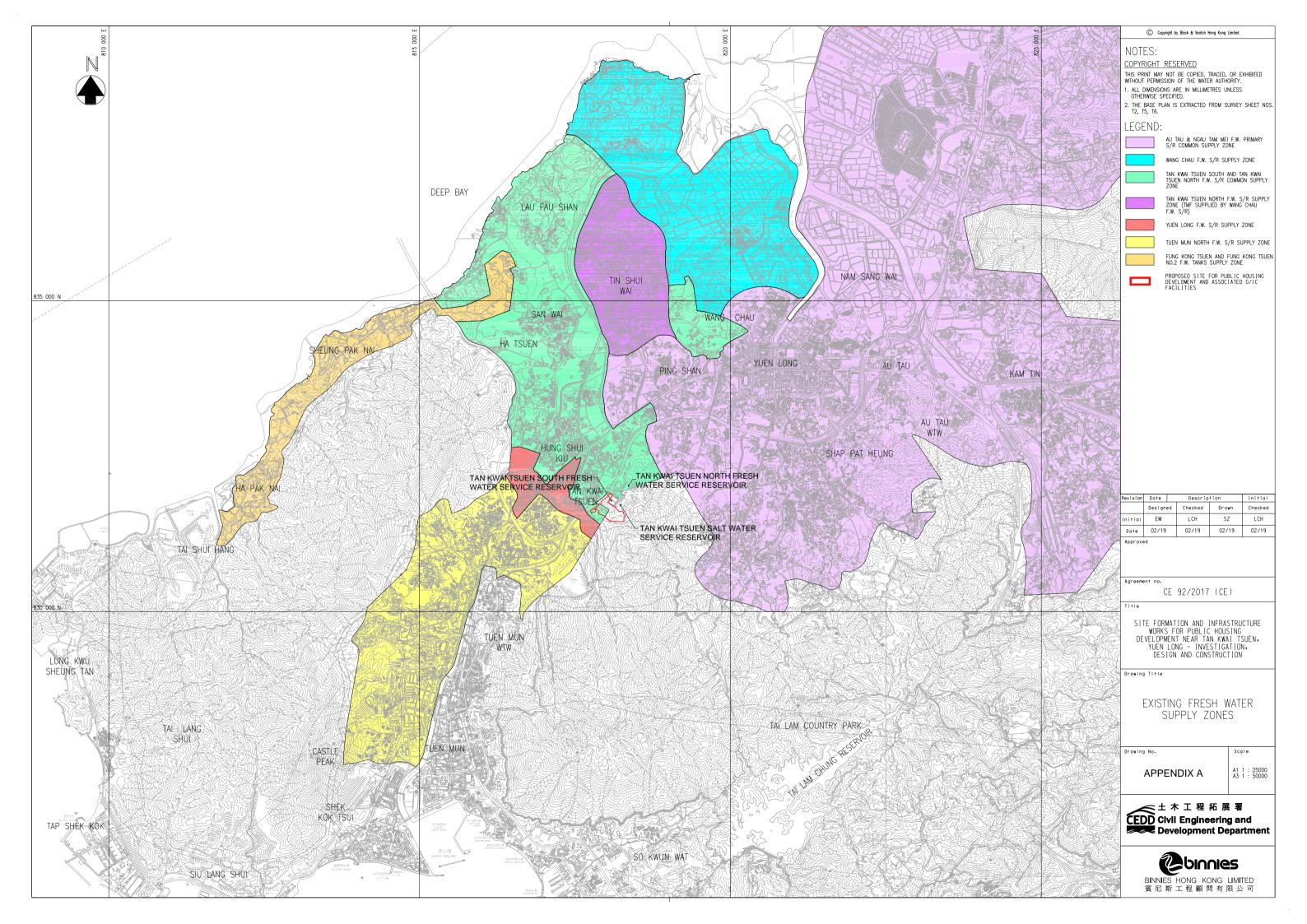
# Appendix A

FRESH WATER SUPPLY ZONE

# **Appendix B**

**SALT WATER SUPPLY ZONE** 



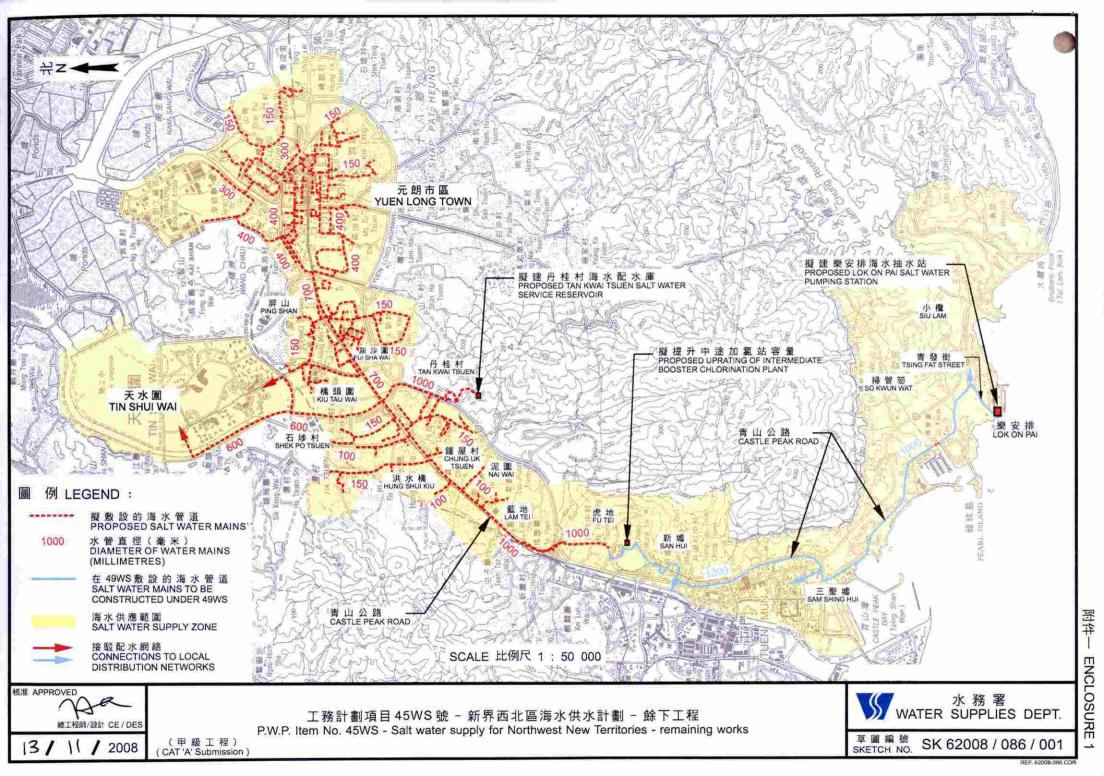


Agreement No. CE 92/2017 (CE)
Site Formation and Infrastructure Works for Public Housing
Development near Tan Kwai Tsuen, Yuen Long
- Investigation, Design and Construction

Final Waterworks Impact Assessment Report for S16 Planning Application (Intensification Scheme) 199086/BIN/089/Issue 3

## **APPENDICES**





## **Appendix C**

WATER DEMANDS, SERVICE RESERVOIR VOLUME AND RESIDUAL HEADS CHECK



## CE 92/2017 (CE) Public Housing Development near Tan Kwai Tsuen, Yuen Long – Investigation, Design and Construction Appendix C - Water Demands, Service Reservoir Volume and Residual Heads Check

### (1) Development Breakdown- Estimate of Water Demand

### Phase 1 (SSF)

			No. of too ditoo	Fresh Wa	ater Unit Demand (n	ո³/h/d) <sup>(6)</sup>	Fresh Water	Flushin	g Water Unit Deman	d (m³/h/d) <sup>(6)</sup>	Flushing Water
Land Use	Domestic Population (1)	Non-domestic population (2)(3)	No. of teaching staffs and students (4) (5) (6)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)
Domestic	5,319	-	-	0.23	0.04	1	1436.1	0.07	-	•	372.3
Non-Domestic	-	168	-	-	-	1	-	-	-	-	-
Retail <sup>(7)</sup>	-	0	-	-	-	-	-	-	0.07	-	0.0
Welfare Facilities (8)	-	152	-	-	0.2	-	30.4	-	0.07	-	10.6
Ancillary Facilities (9)	-	11	-	-	0.04	-	0.4	-	0.07	-	0.8
Others <sup>(9)</sup>	-	5	-	-	0.04	1	0.2	-	0.07	-	0.4
Kindergarten	-	-	201	-	-	0.025	5.0	-	-	0.03	6.0
Total FWMDD (m <sup>3</sup> /day)	tal FWMDD (m³/day)						1,472				390

#### Phase 2 (PRH)

THOSE E (TRITI											
			No of tooching	Fresh W	ater Unit Demand (n	n <sup>3</sup> /h/d) <sup>(6)</sup>	Fresh Water	Flushin	g Water Unit Deman	d (m³/h/d) <sup>(6)</sup>	Flushing Water
Land Use	Domestic Population <sup>(1)</sup>	Non-domestic population (2) (3)	No. of teaching staffs and students (4) (5) (6)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m <sup>3</sup> /d)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m <sup>3</sup> /d)
Domestic	6,480	-	-	0.23	0.04	-	1749.6	0.07	-	-	453.6
Non-Domestic	-	286	-	-	-	-	-	-	-	-	-
Retail <sup>(7)</sup>	-	110	-	-	-	-	-	-	0.07	-	7.7
Welfare Facilities (8)	-	152	-	-	0.2	-	30.4	-	0.07	-	10.6
Ancillary Facilities (9)	-	24	-	-	0.04	-	1.0	-	0.07	-	1.7
Others <sup>(9)</sup>	-	0	-	-	0.04	-	0.0	-	0.07	-	0.0
Kindergarten	-	-	201	-	-	0.025	5.0	-	-	0.03	6.0
Total FWMDD (m <sup>3</sup> /day)							1,786				479

#### Phase 3 (PRH)

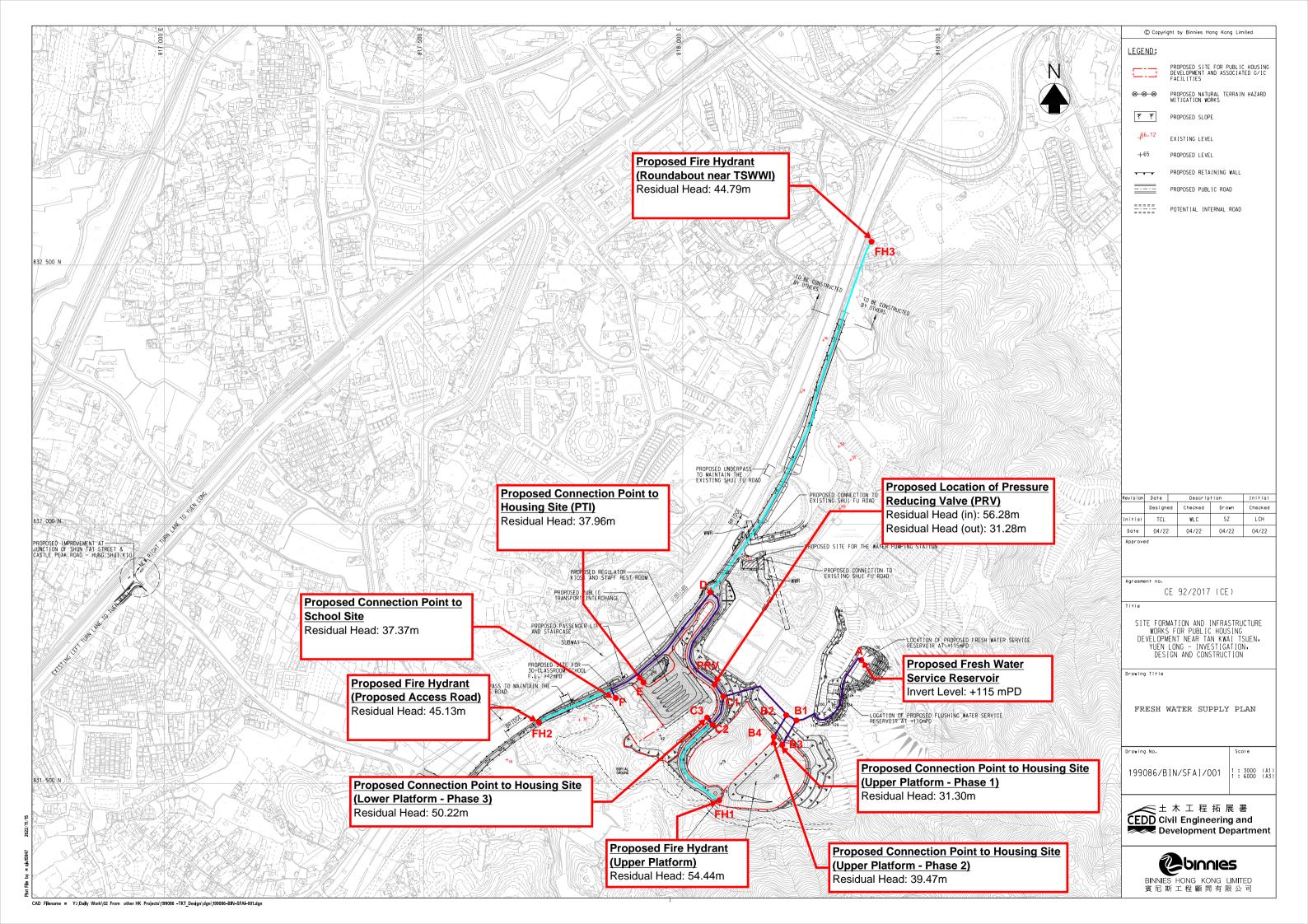
			No oftendine	Fresh W	ater Unit Demand (n	n³/h/d) <sup>(6)</sup>	Fresh Water	Flushin	g Water Unit Demar	ıd (m³/h/d) <sup>(6)</sup>	Flushing Water
Land Use	Domestic Population <sup>(1)</sup>	Non-domestic population (2) (3)	No. of teaching staffs and students (4) (5) (6)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)
Domestic	8,235		-	0.23	0.04		2223.5	0.07	-	-	576.5
Non-Domestic	-	543	-	-	-	-	-	-	-	-	-
Retail <sup>(7)</sup>	-	98	-	-	-	-	-	-	0.07	-	6.9
Welfare Facilities (8)	-	220	-	-	0.2	-	44.0	-	0.07	-	15.4
Ancillary Facilities (9)	-	3	-	-	0.04	-	0.1	-	0.07	-	0.2
Others <sup>(9)</sup>	-	222	-	-	0.04	-	8.9	-	0.07	-	15.5
Kindergarten	-	-	0	-	-	0.025	0.0	-	-	0.03	0.0
Total FWMDD (m³/day)							2,276				614

#### **Primary School**

			No. of Localities	Fresh Wa	ater Unit Demand (n	ո³/h/d) <sup>(6)</sup>	Fresh Water	Flushing	g Water Unit Deman	d (m³/h/d) <sup>(6)</sup>	Flushing Water
Land Use	Domestic Population <sup>(1)</sup>	Non-domestic population (2) (3)	No. of teaching staffs and students <sup>(4) (5) (6)</sup>	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)	Residential	Service Trade	School	Mean Daily Demand (MDD) (m³/d)
Primary School	-	-	821	-	-	0.025	20	-	-	0.03	25

Remarks: (1) Assumed 2.7 persons per flat;

- (2) No water demand is assumed for car parking area;
- (3) Worker densities of 3.5 employees (Retail Trade) and 3.3 employees (Community, Social & Personal Services) per 100 m<sup>2</sup>
  GFA is adopted in accordance with "Commercial and Industrial Floor Space Utilization Survey" issued by the PlanD in 2004/2005;
- (4) Assuming 180 students per kindergarden (with 2 kindergartens planned) and 25.5 students per class for the planned primary school (30-classrooom) with reference to Chapter 3 of the HKPSG;
- (5) Pupil-teacher ratios of 8.6:1 (kindergarden) and 13.8:1 (primary school) are assumed based on Education Bureau's 2017/18 Figures and Statistics;
- (6) Refereces have been taken from the mean unit daily demands recommended in Table 1 and 2 in the WSD DI 1309 and derived from GESF published by EPD;
- (7) The fresh water unit demand of retail is included in domestic service trade;
- (8) The fresh water unit demand of welfare facilities is adopted in accordance with Section 4.4(d) of GESF published by EPD;
- (9) Ancillary facilities and others include office, workshop, PTI and covered walkway etc.



Agreement No. CE 92/2017 (CE)
CE 92/2017 (CE) Public Housing Development near Tan Kwai Tsuen, Yuen Long – Investigation, Design and Construction
Water Supply Impact Assessment
(4) Residual Head Adequacy – Fresh Water Supply

#### Summary of Fresh Water Demand

			Daily 0	peration	Fire Fighti	ng Scenario
			Demand	Peak Demand	Demand	Peak Demand
Supply Zone	Type	MDD (m <sup>3</sup> /day)	Multiplier	(m <sup>3</sup> /day)	Multiplier	(m <sup>3</sup> /day)
Proposed Housing Development (Phase 1)	FW	1,472	3	4,416	1	7,472
Proposed Housing Development (Phase 2)	FW	1,786	3	5,358	1	1,786
Proposed Housing Development (Phase 3)	FW	2,276	3	6,828	1	2,276
Primary School	FW	20	3	60	1	20
Fire Fighting Flow	FW	6.000		-	1	6.000

#### Hydraulic Assessment of Fresh Water Mains (Daily Operation)

Start	End	Nominal Pipe Diameter (mm)	Internal Pipe Diameter (mm)	Available Peak Flow Capacity (m <sup>3</sup> /day)	Pipe Length (m)	Say Pipe Length (m)	Hazen-Williams Coefficient	Peak Demand (m³/day)	Peak Velocity (m/s)	Friction Loss <sup>(1)</sup>	Minor Loss <sup>(2)</sup>	Total Head Loss	Culmulative Head Loss	Elevation (m)	Residual Head (m)	Residual Head Check (>20m)	Remarks
E	F	250	250	6362	92	120	110	60	0.01	0.00	0.00	0.00	1.49	42.00	46.51	OK	
D	Е	250	250	6362	235	290	110	60	0.01	0.00	0.00	0.00	1.49	43.00	45.51	OK	
PRV	D	300	300	9161	220	270	110	60	0.01	0.00	0.00	0.00	1.49	49.50	39.01	OK	
C1	PRV	300	300	9161	100	120	110	60	0.01	0.00	0.00	0.00	1.49	55.00	33.51	OK	After PRV
CI	1100	300	300	7101	100	120	110	00	0.01	0.00	0.00	0.00	1.17	33.00	58.51	OK	
C2	C3	150	150	2290	14	20	110	6828	4.47	3.36	0.67	4.03	5.41	57.50	52.09	OK	
C1	C2	300	300	9161	56	70	110	6828	1.12	0.40	0.08	0.48	8.07	58.50	48.43	OK	
B2	C1	450	450	20612	161	200	110	6888	0.50	0.16	0.03	0.19	1.49	59.50	54.01	OK	
B2	B4	150	150	2290	48	60	110	5358	3.51	6.43	1.29	7.72	7.72	75.00	32.28	OK	
B1	B2	450	450	27483	22	30	110	12246	0.89	0.07	0.01	0.08	1.38	88.00	25.62	OK	
B1	В3	150	150	3054	55	70	110	4416	2.89	5.25	1.05	6.30	7.59	82.00	25.41	OK	
A	B1	450	450	27483	215	260	110	16662	1.21	1.08	0.22	1.29	1.29	90.00	23.71	OK	Before PRV

#### Hydraulic Assessment of Fresh Water Mains (Fire Fighting)

Sta	ırt I	End	Nominal Pipe Diameter (mm)	Internal Pipe Diameter (mm)	Available Peak Flow Capacity (m³/day)	Pipe Length (m)	Say Pipe Length (m)	Hazen-Williams Coefficient	Peak Demand (m3/day) <sup>(3)</sup>	Peak Velocity (m/s)	Friction Loss <sup>(1)</sup>	Minor Loss <sup>(2)</sup>	Total Head Loss	Culmulative Head Loss	Elevation (m)	Residual Head (m)	Residual Head Check (>17m for draw off note and >20m for others)	Remarks
Ε	) I	FH3	250	250	6362	942	1140	110	6000	1.41	12.52	2.50	15.02	20.21	25.00	44.79	OK	
F	7 I	FH2	250	250	6362	136	170	110	6000	1.41	1.87	0.37	2.24	12.87	32.00	45.13	OK	
E	3	F	250	250	6362	92	120	110	6020	1.42	1.33	0.27	1.59	10.63	42.00	37.37	OK	After PRV
Ε	)	E	250	250	6362	235	290	110	6020	1.42	3.20	0.64	3.85	9.04	43.00	37.96	OK	Aitellity
PF	V.	D	300	300	9161	220	270	110	6020	0.99	1.23	0.25	1.47	5.19	49.50	35.31	OK	
С	1   1	PRV	300	300	9161	100	120	110	6020	0.99	0.55	0.11	0.65	3.72	55.00	31.28	OK	
·		1 100	300	300	7101	100	120	110	0020	0.77	0.55	0.11	0.03	5.72	33.00	56.28	OK	
C	2 I	FH1	300	300	9161	205	250	110	6020	0.99	1.14	0.23	1.36	3.06	57.50	54.44	OK	
C	2	C3	150	150	2290	14	20	110	8276	5.42	4.79	0.96	5.75	6.28	58.50	50.22	OK	
C	1	C2	300	300	9161	56	70	110	8276	1.36	0.57	0.11	0.69	2.17	59.50	53.33	OK	
В	2	C1	450	450	20612	161	200	110	8296	0.60	0.23	0.05	0.27	1.70	61.00	52.30	OK	Before PRV
В	2	B4	300	300	9161	48	60	110	7786	1.27	0.44	0.09	0.53	0.53	75.00	39.47	OK	
В	1	B2	450	450	20612	22	30	110	10082	0.73	0.05	0.01	0.06	1.49	88.00	25.51	OK	
В	1	В3	300	300	9161	55	70	110	13472	2.21	1.41	0.28	1.70	1.70	82.00	31.30	OK	
A	1	B1	450	450	27483	215	260	110	17554	1.28	1.19	0.24	1.43	1.43	90.00	23.57	OK	

Note:

(1) By Hazen Williams Equation, Friction Loss =  $\frac{10.67L(Q^{1.85})}{C^{1.852}d^{482}}$  where L is length of pipeline (m), C is Hazen-Williams Coefficient, d is internal diameter of pipe (m) and Q is design flow of pipe (m<sup>3</sup>/s)

(2) Assume minor loss (i.e. due to bends and tees) is 20% of the friction loss.

(3) Assume Pressure Reducing Valve (PRV) has lowered the Residual Head by 30m.

Agreement No. CE 92/2017 (CE)
CE 92/2017 (CE) Public Housing Development near Tan Kwai Tsuen, Yuen Long – Investigation, Design and Construction

Water Supply Impact Assessment

(5) Residual Head Adequacy - Flushing Water Supply

#### Summary of Flushing Water Demand

			Daily O	peration
		_	Demand	Peak Demand
Supply Zone	Type	MDD (m <sup>3</sup> /day)	Multiplier	(m <sup>3</sup> /day)
Proposed Housing Development (Phase 1)	FLW	390	2	780
Proposed Housing Development (Phase 2)	FLW	479	2	958
Proposed Housing Development (Phase 3)	FLW	614	2	1,228
Primary School	FI.W	25	2	50

#### Hydraulic Assessment of Flushing Water Mains

Invert level of the proposed FLWSR

110 mPD

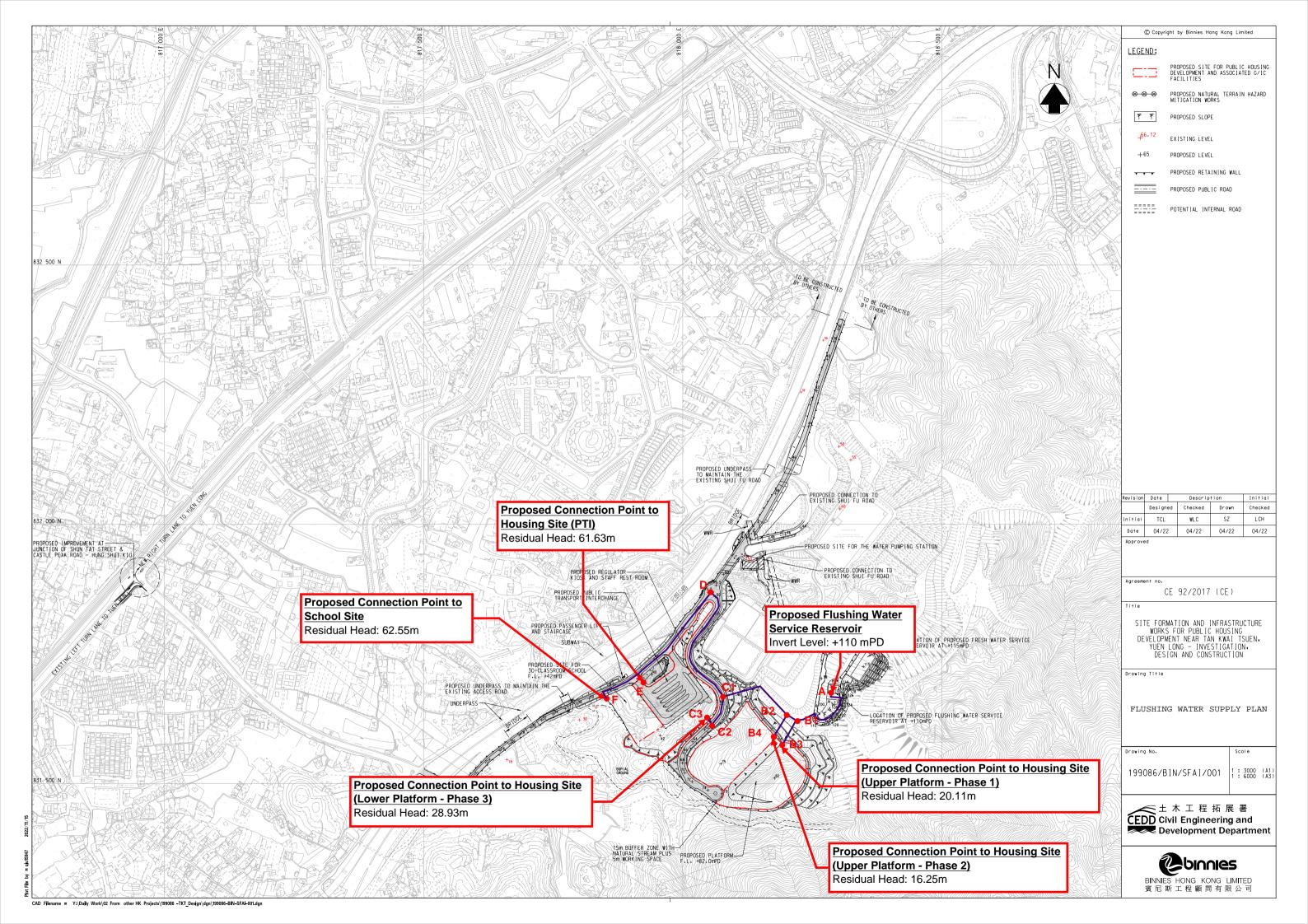
Start	End	Nominal Pipe Diameter (mm)	Internal Pipe Diameter (mm)	Available Peak Flow Capacity (m³/day)	Pipe Length (m)	Say Pipe Length (m)	Hazen-Williams Coefficient	Peak Demand (m³/day)	Peak Velocity (m/s)	Friction Loss <sup>(1)</sup>	Minor Loss <sup>(2)</sup>	Total Head Loss	Culmulative Head Loss	Elevation (m)	Residual Head (m)	Residual Head Check (>15m)
E	F	80	80	651	92	120	90	50	0.12	0.07	0.01	0.08	5.45	42.00	62.55	OK
D	E	80	80	651	235	290	90	50	0.12	0.17	0.03	0.20	5.37	43.00	61.63	OK
C1	D	80	80	651	246	300	90	50	0.12	0.17	0.03	0.21	5.16	49.50	55.34	OK
C2	C3	80	80	651	14	20	90	1228	2.83	4.35	0.87	5.22	23.97	57.50	28.53	OK
C1	C2	80	80	651	56	70	90	1228	2.83	15.22	3.04	18.26	22.57	58.50	28.93	OK
B2	C1	200	200	4072	161	200	90	1278	0.47	0.54	0.11	0.65	4.95	59.50	45.55	OK
B2	B4	80	80	651	48	60	90	1008	2.32	9.05	1.81	10.86	18.75	75.00	16.25	OK
B1	B2	200	200	4072	22	30	90	1788	0.66	0.15	0.03	0.18	4.31	88.00	17.69	OK
B1	В3	80	80	651	55	70	90	780	1.80	6.57	1.31	7.89	7.89	82.00	20.11	OK
A	B1	200	200	4072	215	260	90	3016	1.11	3.44	0.69	4.12	4.12	90.00	15.88	OK

Note:

(1) By Hazen Williams Equation, Friction Loss =

 $\frac{10.67L(Q^{185})}{C^{1852}d^{1852}} \quad \text{where L is length of pipeline (m), C is Hazen-Williams Coefficient, d is internal diameter of pipe (m) and Q is design flow of pipe (m^3/s)}$ 

(2) Assume minor loss (i.e. due to bends and tees) is 20% of the friction loss.



Agreement No. CE 92/2017 (CE)
CE 92/2017 (CE) Public Housing Development near Tan Kwai Tsuen, Yuen Long - Investigation, Design and Construction Water Supply Impact Assessment
(5) Residual Head Adequacy - Flushing Water Supply

#### Summary of Flushing Water Demand

			Daily O	peration
			Demand	Peak Demand
Supply Zone	Type	MDD (m <sup>3</sup> /day)	Multiplier	(m <sup>3</sup> /day)
Proposed Housing Development (Phase 1)	FLW	390	2	780
Proposed Housing Development (Phase 2)	FLW	479	2	958
Proposed Housing Development (Phase 3)	FLW	614	2	1,228
Primary School	FLW	25	2	50

### Hvdraulic Assessment of Flushing Water Mains

Invert level of the proposed FLWSR

110 mPD

Start	End	Nominal Pipe Diameter (mm)	Internal Pipe Diameter (mm)	Available Peak Flow Capacity (m³/day)	Pipe Length (m)	Say Pipe Length (m)	Hazen-Williams Coefficient	Peak Demand (m³/day)	Peak Velocity (m/s)	Friction Loss <sup>(1)</sup>	Minor Loss <sup>(2)</sup>	Total Head Loss	Culmulative Head Loss	Elevation (m)	Residual Head (m)	Residual Head Check (>15m)
E	F	80	80	651	92	120	90	50	0.12	0.07	0.01	0.08	5.45	42.00	62.55	OK
D	E	80	80	651	235	290	90	50	0.12	0.17	0.03	0.20	5.37	43.00	61.63	OK
C3	D	80	80	651	246	300	90	50	0.12	0.17	0.03	0.21	5.16	49.50	55.34	OK
C2	C3	80	80	651	14	20	90	1228	2.83	4.35	0.87	5.22	23.97	57.50	28.53	OK
C1	C2	80	80	651	56	70	90	1228	2.83	15.22	3.04	18.26	22.57	58.50	28.93	OK
B2	C1	200	200	4072	161	200	90	1278	0.47	0.54	0.11	0.65	4.95	59.50	45.55	OK
B2	B4	80	80	651	48	60	90	1008	2.32	9.05	1.81	10.86	18.75	75.00	16.25	OK
B1	B2	200	200	4072	22	30	90	1788	0.66	0.15	0.03	0.18	4.31	88.00	17.69	OK
B1	В3	80	80	651	55	70	90	780	1.80	6.57	1.31	7.89	7.89	82.00	20.11	OK
A	B1	200	200	4072	215	260	90	3016	1.11	3.44	0.69	4.12	4.12	90.00	15.88	OK

Note:

(1) By Hazen Williams Equation, Friction Loss =

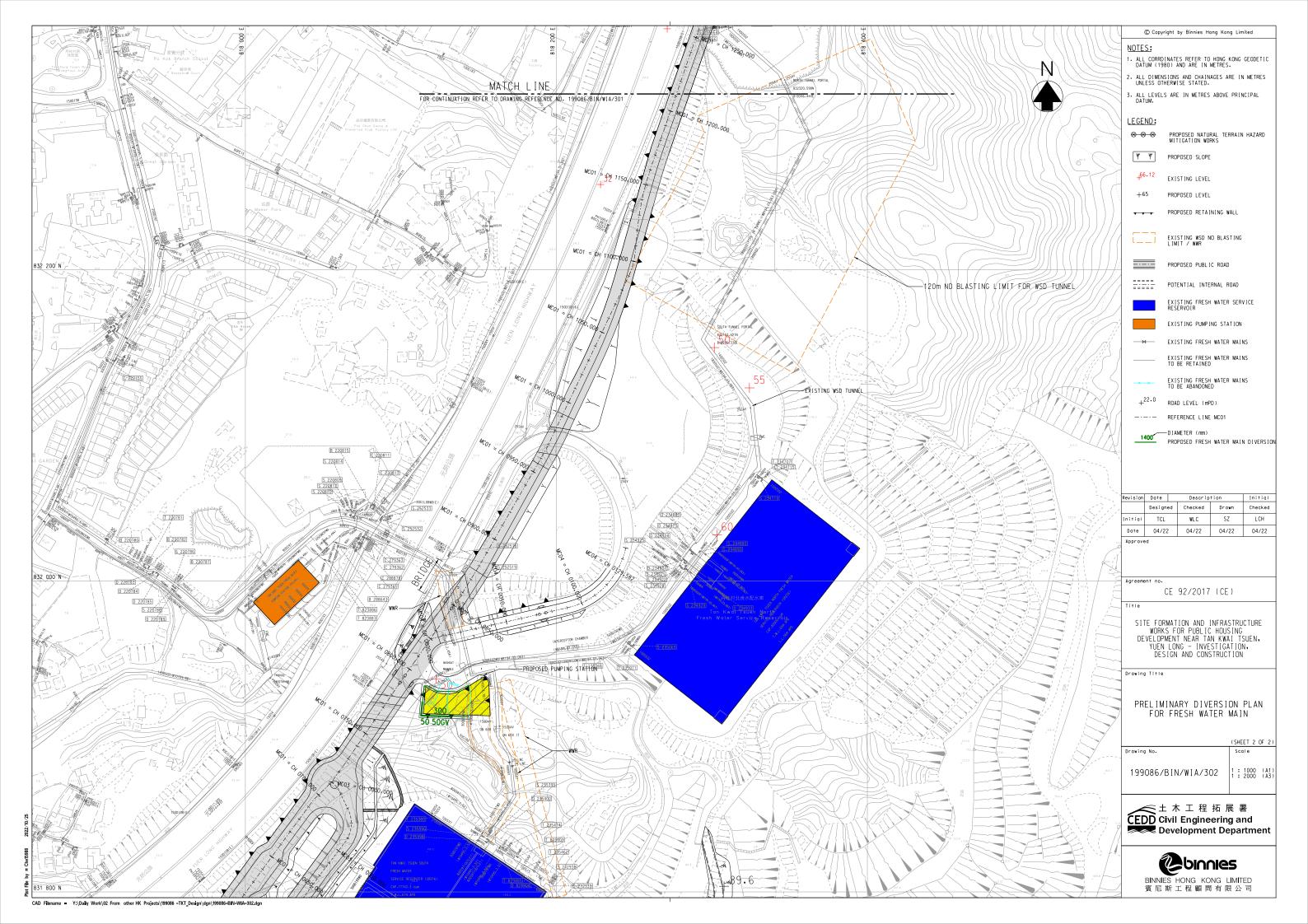
 $\frac{10.67L(Q^{1.85})}{C^{1.852}d^{4.82}} \quad \text{where L is length of pipeline (m), C is Hazen-Williams Coefficient, d is internal diameter of pipe (m) and Q is design flow of pipe (m^3/s)}$ 

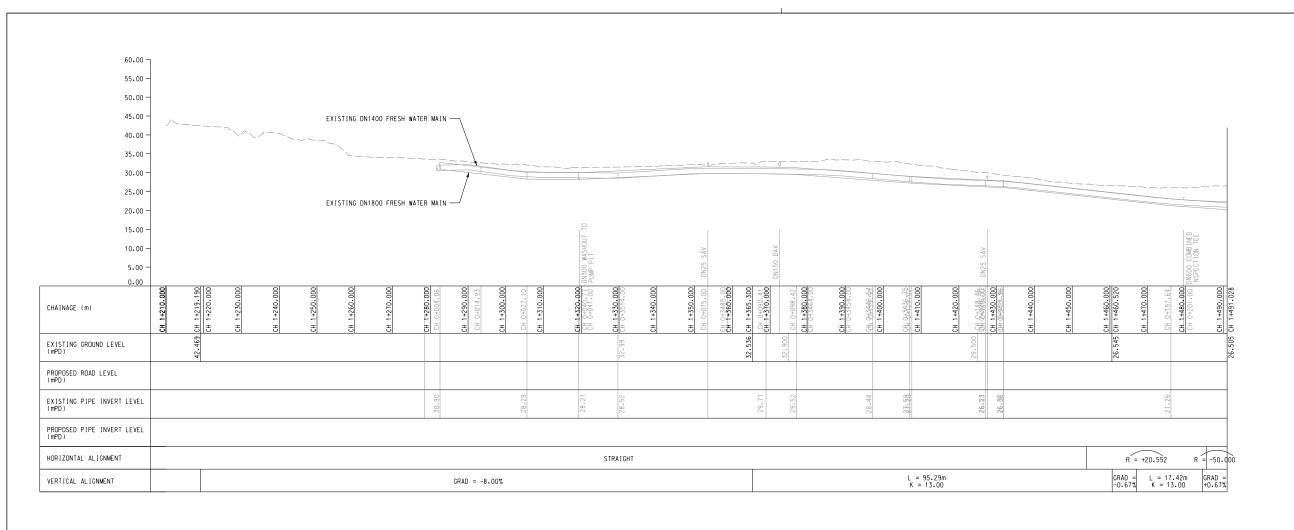
(2) Assume minor loss (i.e. due to bends and tees) is 20% of the friction loss.

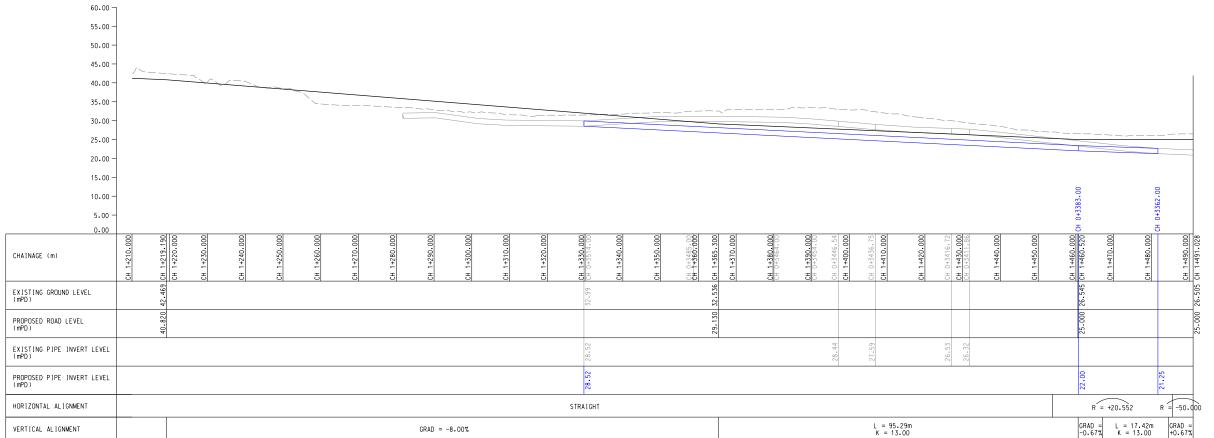
# Appendix D

## PROPOSED DIVERSION PLAN FOR EXISTING WATER MAINS









PROPOSED ROAD LONGITUDINAL PROFILE (MCO1)

SCALE A1 1 : 500
A3 1 : 1000

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### NOTES.

- 1. ALL DIMENSIONS AND CHAINAGES ARE IN METRES UNLESS OTHERWISE STATED.
- 2. ALL LEVELS ARE IN METRES ABOVE PRINCIPAL DATUM.
- 3. ALL VERTICAL CURVES ARE PARABOLIC. ALL HORIZONTAL TRANSITIONS ARE CLOTHOIDAL.
- 4. CHAINAGES AND LEVELS SHOWN ARE ALONG THE SETTING OUT LINES.
- 5. EXISTING GROUND LEVELS SHOWN ARE APPROXMATE ONLY.

### LEGEND

— — — — EXISTING PROFILE

- PROPOSED PROFILE

EXISTING FRESH WATER MAIN

PROPOSED FRESH WATER MAIN DIVERSION

Revision	Date		Descrip	Initial	
	Designe	bd	Checked	Drawn	Checked
Initial	KWM		KWM LCH		LCH
Date	03/21		03/21	03/21	03/21

### Approved

Agreement no.

CE 92/2017 (CE)

Title

SITE FORMATION AND INFRASTRUCTURE WORKS FOR PUBLIC HOUSING DEVELOPMENT NEAR TAN KWAI TSUEN, YUEN LONG - INVESTIGATION, DESIGN AND CONSTRUCTION

Drawing Title

PRELIMINARY DIVERSION PLAN FOR FRESH WATER MAIN-LONGITUDINAL SECTION

Drawing No.

30016

199086/BIN/WIA/304

土木工程拓展署
CEDD Civil Engineering and
Development Department

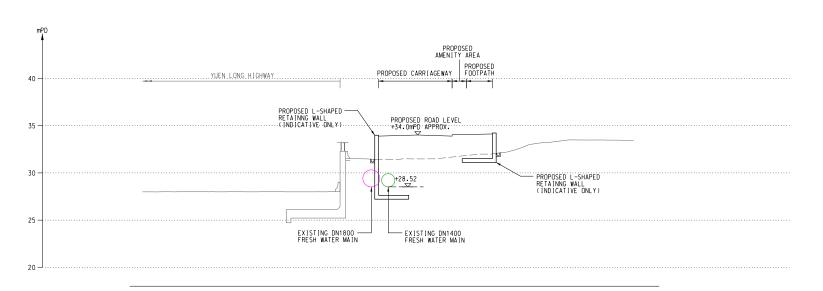




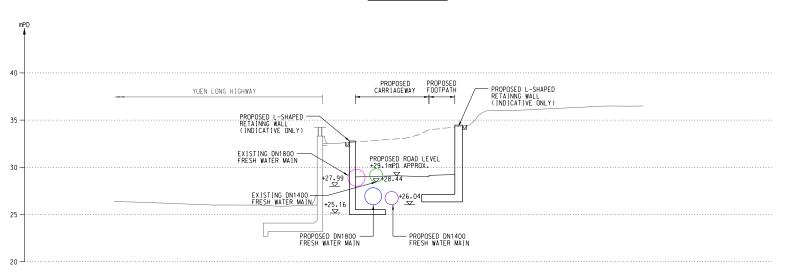
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### NOTES:

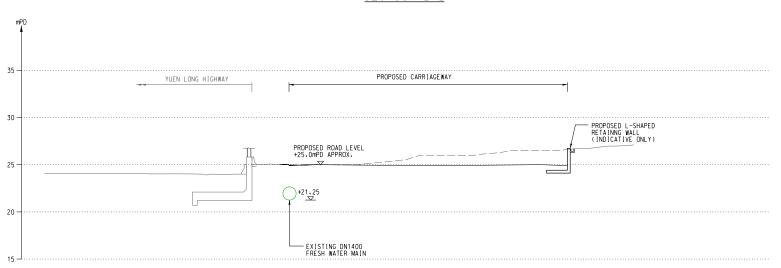
- 1. ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHER WISE STATED.
- 2. ALL LEVELS ARE IN METRES ABOVE PRINCIPAL DATUM (P.D.).
- 3. THIS DRAWING SHALL BE READ IN CONJUNCTION
  WITH OTHER RELEVANT DRAWINGS. SPECIFICATION
  AND INSTRUCTIONS ISSUED BY THE ENGINEED



### SECTION A-A



### SECTION B-B



### SECTION C-C



### proved

Agreement no.

CE 92/2017 (CE)

Title

SITE FORMATION AND INFRASTRUCTURE WORKS FOR PUBLIC HOUSING DEVELOPMENT NEAR TAN KWAI TSUEN, YUEN LONG - INVESTIGATION, DESIGN AND CONSTRUCTION

Drawing Title

SITE FORMATION PLAN (SECTION A. B & C )

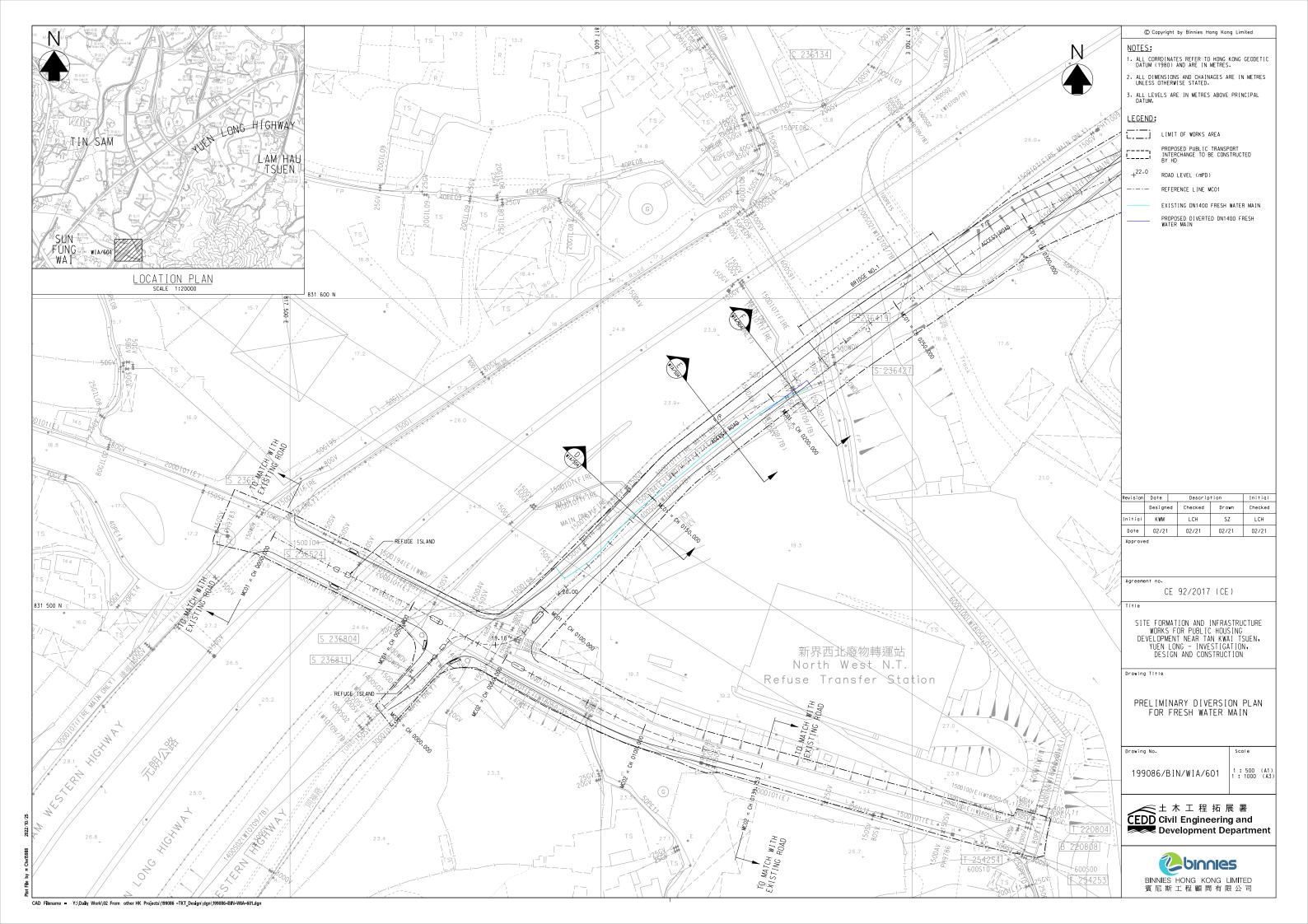
Drawing No.

1 : 200 (A1) 1 : 400 (A3)

199086/BIN/WIA/502

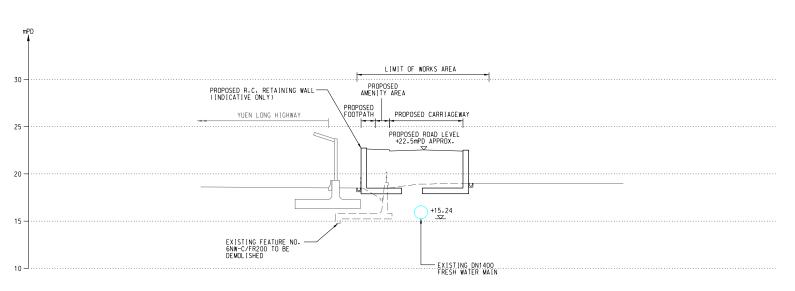




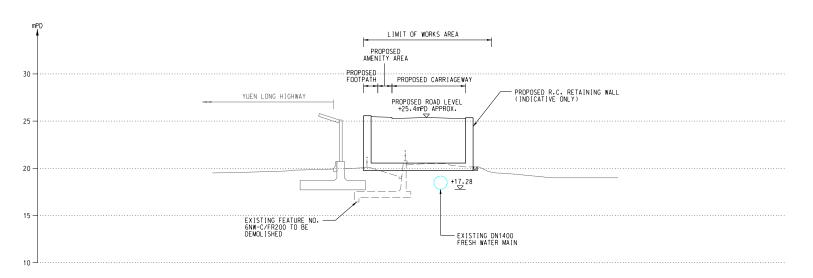


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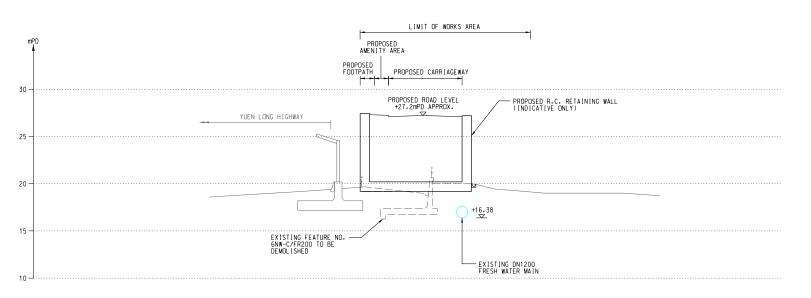
- 1. ALL DIMENSIONS ARE IN MILLIMETRES UNLESS OTHER WISE STATED.
- 2. ALL LEVELS ARE IN METRES ABOVE PRINCIPAL DATUM (P.D.).



### SECTION D-D



### <u>SECTION E-E</u>



SECTION F-F

	Designed	Checked	Drawn	Checked	
Initial	KWM	LCH	SZ	LCH	
Date	11/21	11/21	11/21	11/21	
	•				

Description

Revision Date

### Agreement no.

CE 92/2017 (CE)

SITE FORMATION AND INFRASTRUCTURE WORKS FOR PUBLIC HOUSING DEVELOPMENT NEAR TAN KWAI TSUEN, YUEN LONG - INVESTIGATION, DESIGN AND CONSTRUCTION

Drawing Title

SITE FORMATION PLAN (SECTION D. E & F )

199086/BIN/WIA/602

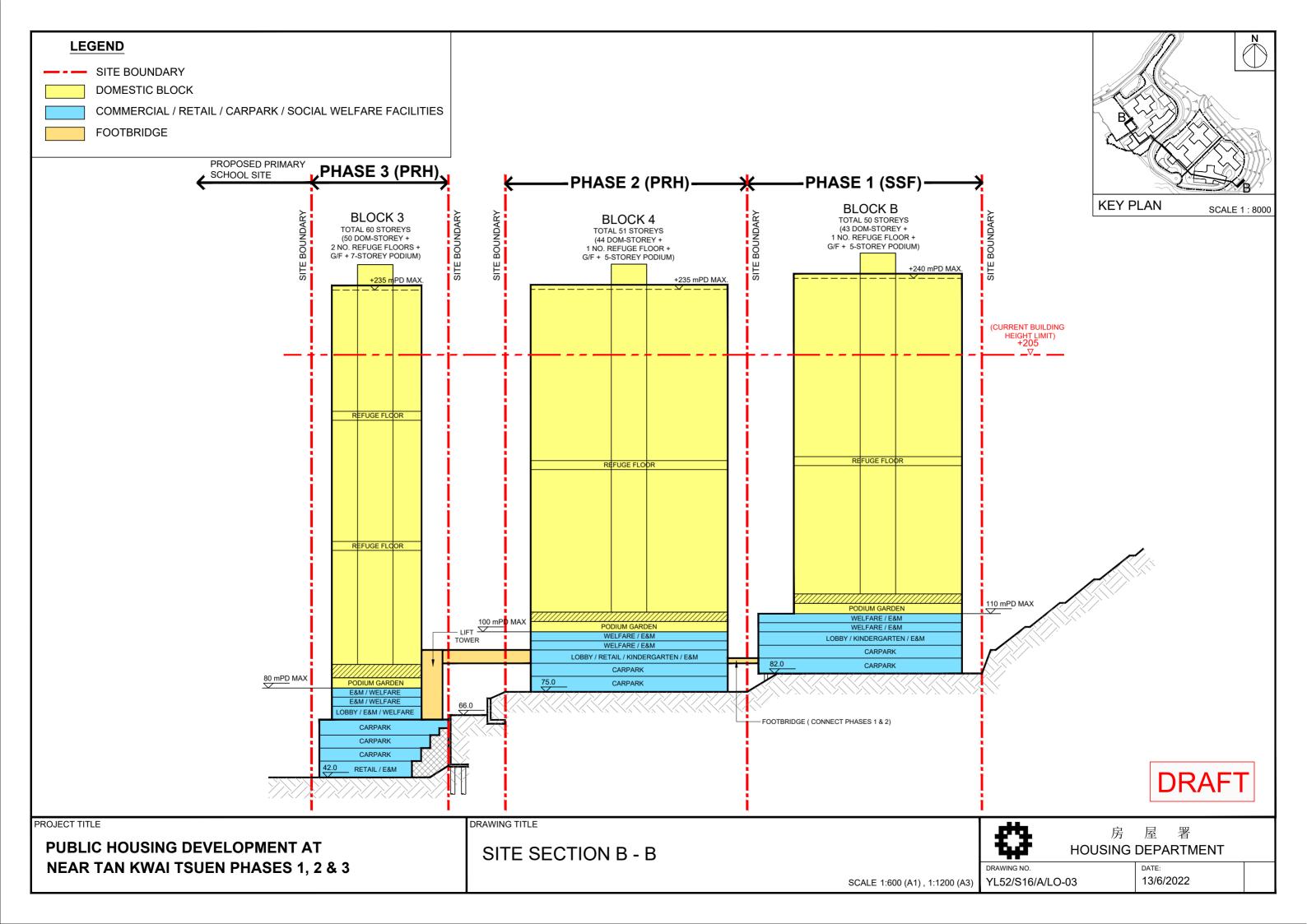
土木工程拓展署 CEDD Civil Engineering and Development Department

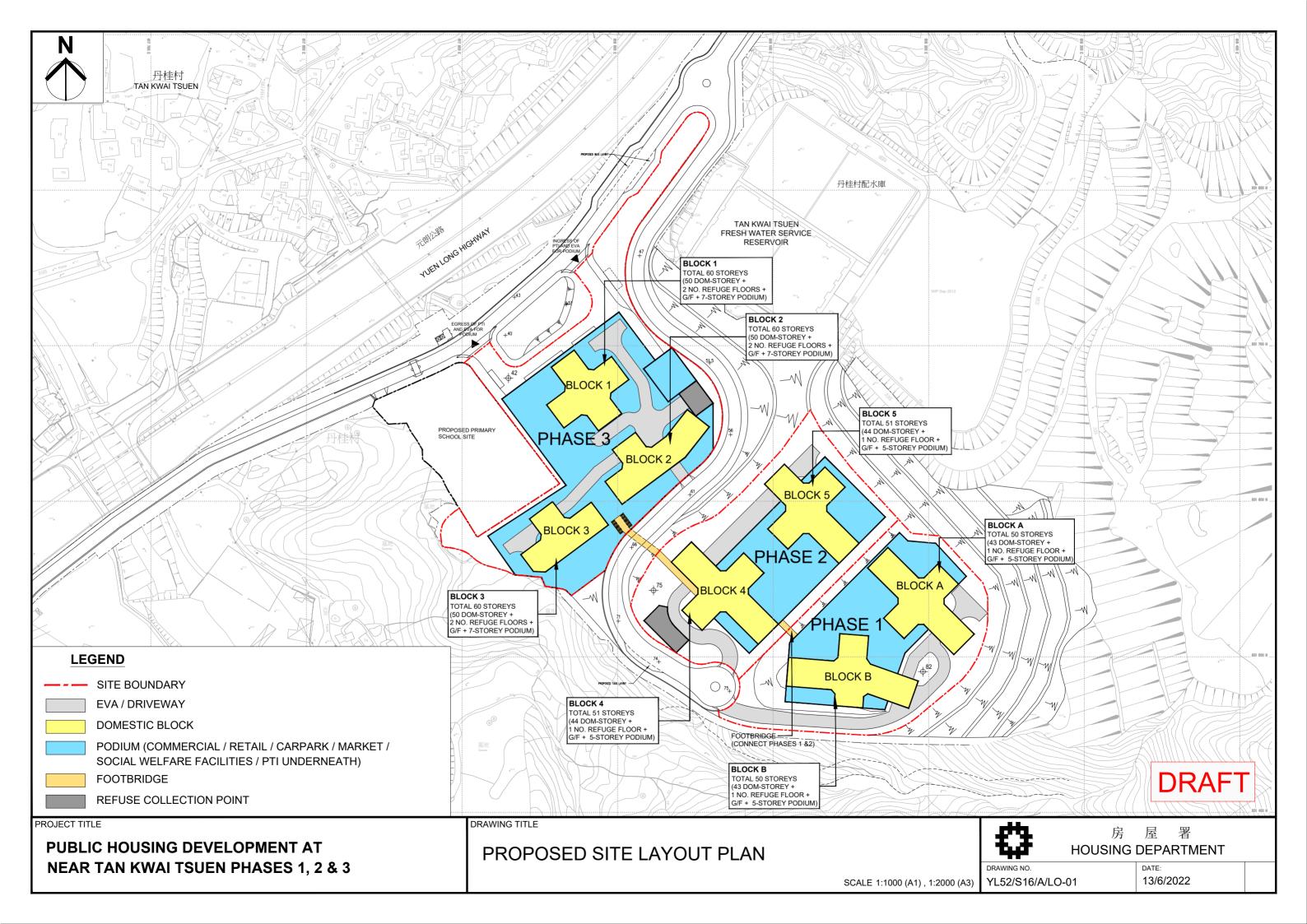


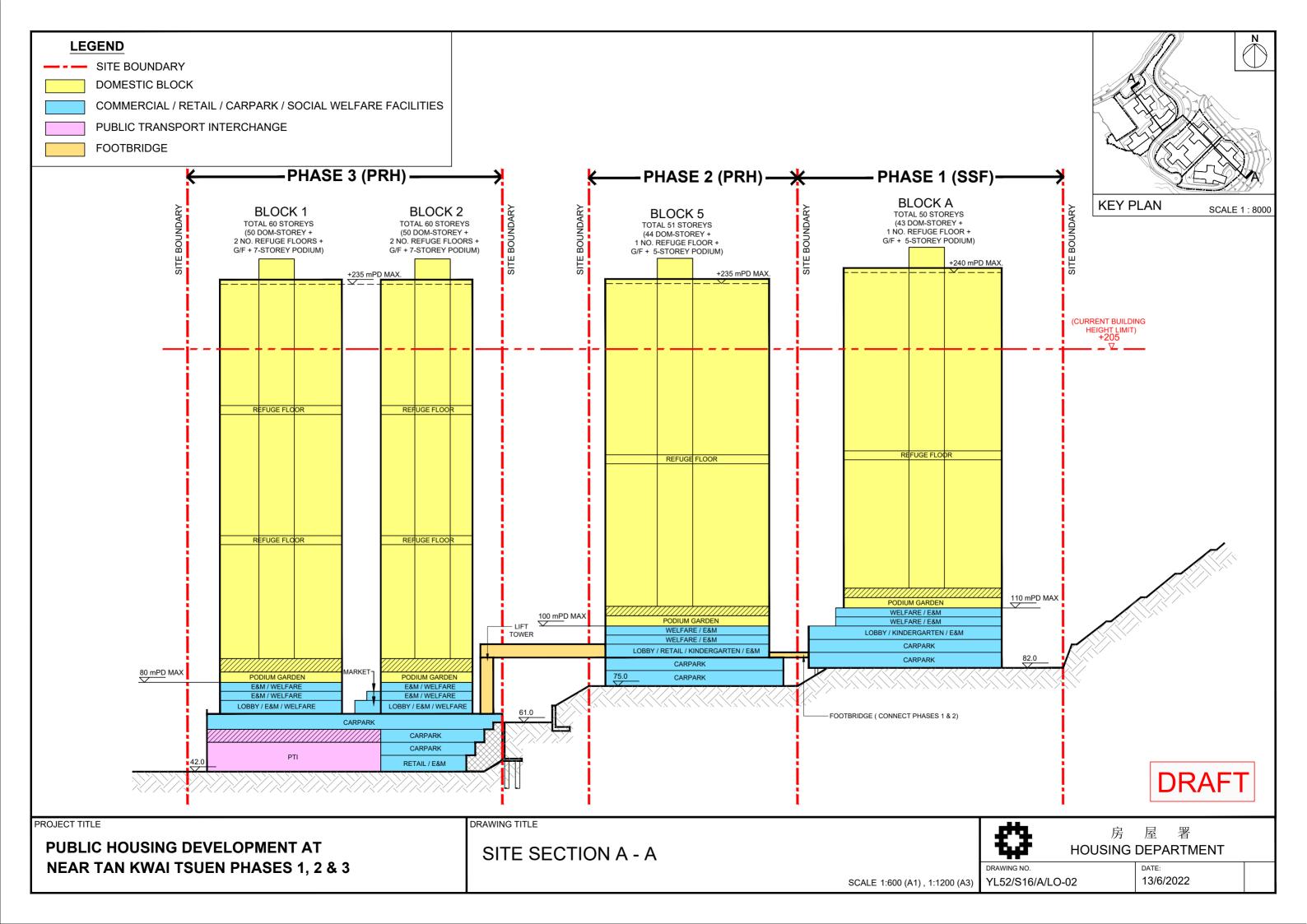
Final Waterworks Impact Assessment Report for S16 Planning Application (Intensification Scheme) 199086/BIN/089/Issue 3

## **Appendix E**

PRELIMINARY HOUSING SITE LAYOUT PLAN





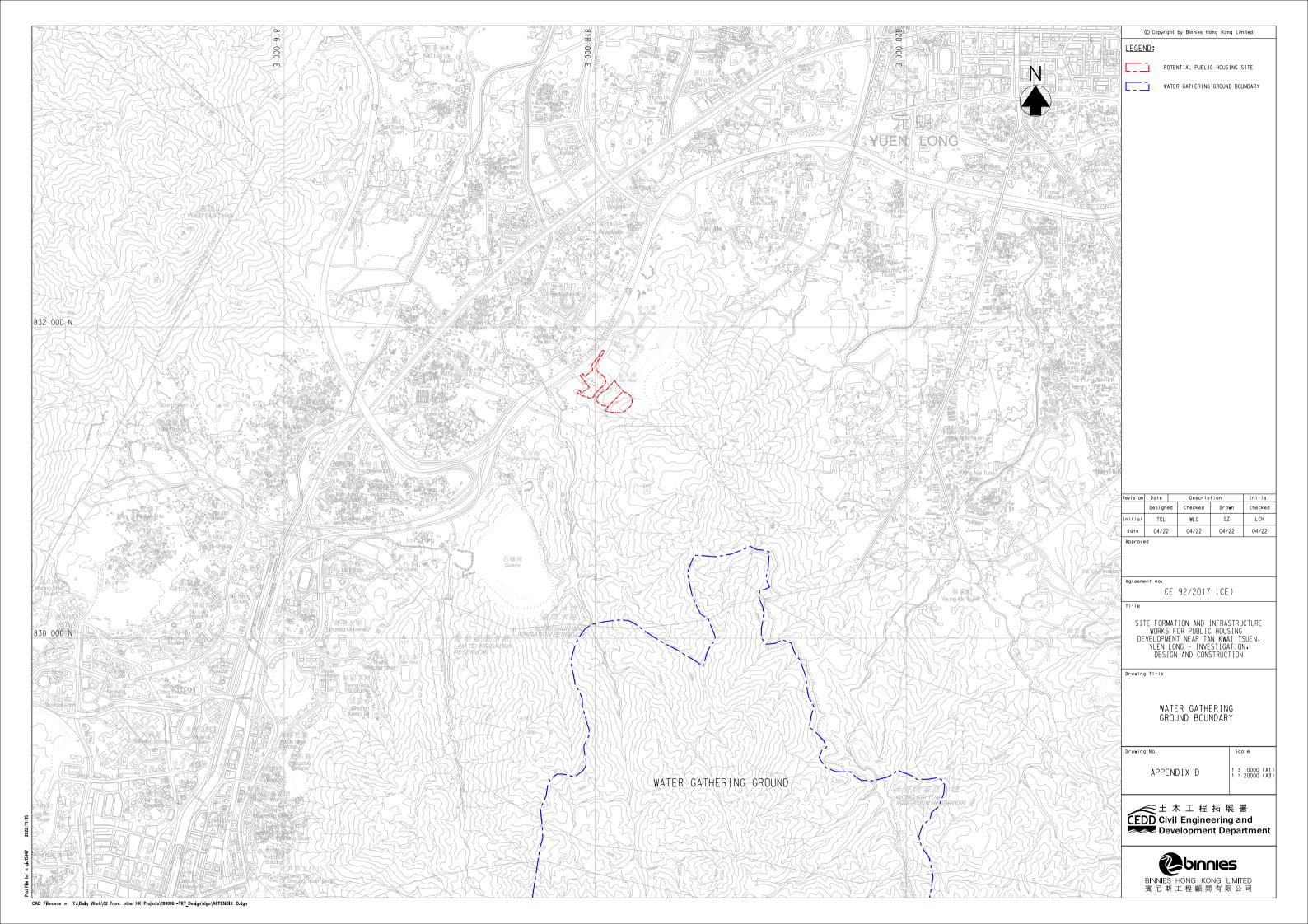


Final Waterworks Impact Assessment Report for S16 Planning Application (Intensification Scheme) 199086/BIN/089/Issue 3

**Appendix F** 

**WATER GATHERING GROUND** 





Final Waterworks Impact Assessment Report for S16 Planning Application (Intensification Scheme) 199086/BIN/089/Issue 3

## Appendix G

**Surge Analysis Report for Water Supply Systems** 



# Calculation for System Curve of Fresh Water

### **Design Assumption**

Project

### 1 Used Code

- 1 Manual of Mainlaying Practice (2012 Edition)
- 2 Stomwater Drainage Manual (2018 Edition)

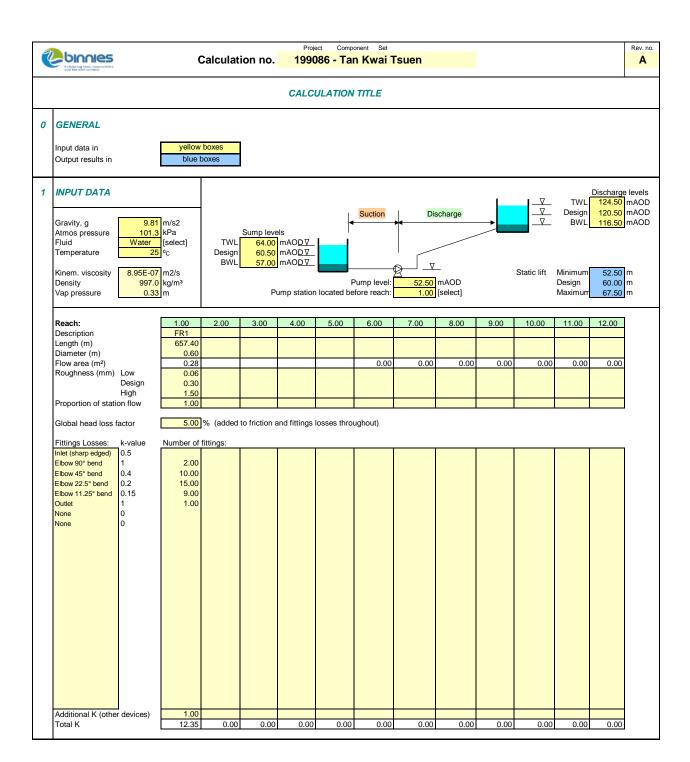
### 2 Design Assumption

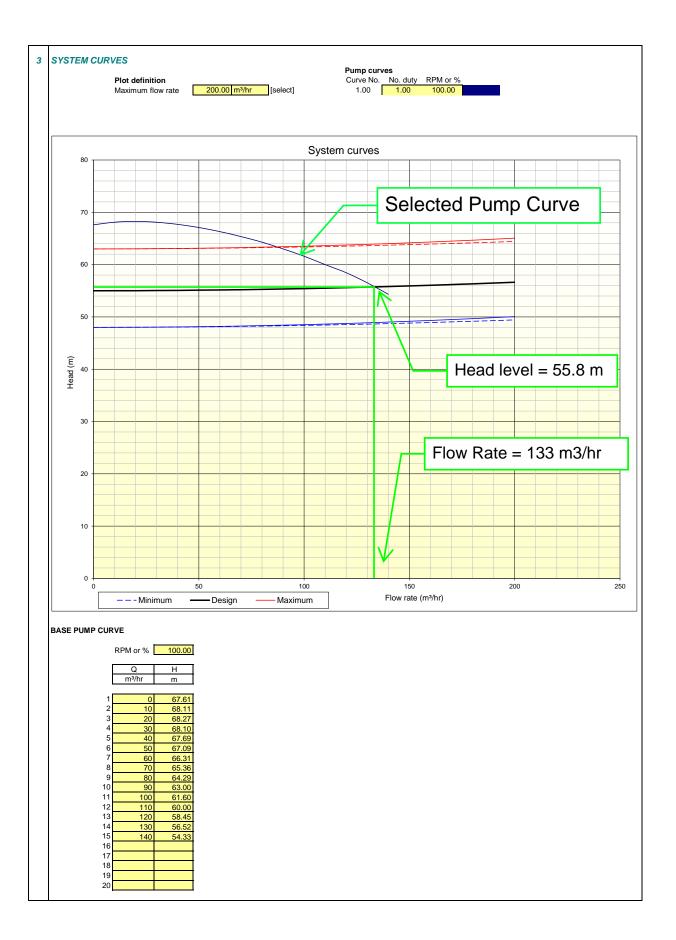
### FLUID PROPERTIES

	salinity	pressure (gauge)	density	kin visc	vapour pre	R	С	То	μο	То	Temp	μ, dyn visc
	(g/l)	(Pa)	(kg/m³)	(m2/s)	(m)	J/(kg.K)	(K)	(K)	(cP)	(oR)	(Kelvin)	(cP)
Water	0	n/a	997.05	0.00	0.33							
Air	n/a	101300	1.18	0.00	0.00	286.90	120.00	291.15	0.02	524.07	298.15	0.02

### 3 Used Equcation

- 1 Flow Rate = Cross-section Area x Average Speed
- 2 Velocity =  $\frac{Flow\ Rate}{Flow\ Area}$ ;
- 3 Velocity Head=  $\frac{Velocity^2}{2 \times Gravity}$ ;
- 4 Reynolds Number =  $\frac{Velocity \ x \ Gravity}{Kinem \ Viscosity}$ ;





### 4 DATA LIBRARY

Fittings K Values

	Item	K
,	None	0.00
Inlet / outlet	Inlet (sharp edged)	0.50
	Outlet	1.00
	Inlet (re-entrant)	0.80
	Inlet (slightly rounded)	0.25
	Inlet (bellmouth)	0.05
Bends	Elbow 90° bend	1.00
	Elbow 45° bend	0.40
	Elbow 22.5° bend	0.20
	Elbow 11.25° bend	0.15
	Short R 90° bend	0.75
	Short R 45° bend	0.30
	Short R 22.5° bend	0.15
	Short R 11.25° bend	0.10
	Long R 90° bend	0.40
	Long R 45° bend	0.20
	Long R 22.5° bend	0.10
	Long R 11.25° bend	0.05
	Sweep 90°	0.20
	Sweep 45°	0.10
	Sweep 22.5°	0.10
	Mitre bend 90°	1.20
	Mitre bend 45°	0.40
	Mitre bend 22.5°	0.20
T W	Mitre bend 11.25°	0.15
Transitions	Taper up (4:5)	0.03
	Taper up (3:4)	0.04
	Taper up (1:2)	0.12
	Expansion 4:5	0.15
	Expansion 3:4	0.20
	Expansion 2:3	0.35
	Expansion 1:2	0.60
	Expansion 1:3	0.80
	Expansion 1:5	1.00
	Contraction 5:4	0.15
	Contraction 4:3	0.20
	Contraction 3:2	0.30
	Contraction 2:1	0.35
	Contraction 3:1	0.45
	Contraction ≥5:1	0.50
T-junctions	T line to branch 90°	1.20
_	T line to branch 45°	0.60
	T line to branch 30°	0.40
	T straight through	0.35
	T line to branch 90° Radiused	0.80
Valves	Gate valve	0.12
	Butterfly valve	0.30
	Globe valve	5.00
	Plug valve	0.25
	Swing check valve	1.00
	Flap valve	1.50
	Foot valve & strainer	2.50
User defined	Other	0.00
OSEI UEIIIEU	Other	0.00
	Other	0.00
l	Other	0.00

Sustam Curus Calculation			
System Curve Calculation			
Gravity	9.81	m/s2	
Water visc	0.00		
Density	997.05	kg/m3	
U/s head	64.00	60.50	57.00 m
D/s head	116.50	120.50	124.50 m
Static lift	52.50	60.00	67.50 m

Section		1.00
Length	m	657.40
Diameter	m	0.45
Area	m2	0.16
k		12.35
Flow factor		1.00

MIN Ks	0.06		Total	Min	Max
Flow (m³/s)	Losses (m)		losses	head	head
		fittings	(m)	(m)	(m)
0 0	0.00	0.00	0.000	48.000	63.000
0.003 10	0.00	0.00	0.005	48.005	63.005
0.006 20	0.00	0.00	0.018	48.018	63.018
0.008 30	0.00	0.00	0.038	48.038	63.038
0.011 40 0.014 50	0.01	0.00	0.065	48.065	63.065
0.014 50 0.017 60	0.01 0.02	0.00 0.01	0.099	48.099	63.099
0.017 00	0.02	0.01	0.141 0.189	48.141 48.189	63.141 63.189
0.022 80	0.03	0.01	0.244	48.244	63.244
0.025 90	0.04	0.02	0.306	48.306	63.306
0.028 100	0.04	0.02	0.374	48.374	63.374
0.031 110	0.05	0.02	0.449	48.449	63.449
0.033 120	0.06	0.03	0.531	48.531	63.531
0.036 130	0.07	0.03	0.620	48.620	63.620
0.039 140	0.08	0.04	0.715	48.715	63.715
0.042 150	0.09	0.04	0.818	48.818	63.818
0.044 160 0.047 170	0.10 0.11	0.05 0.06	0.926 1.042	48.926 49.042	63.926 64.042
0.047 170	0.11	0.06	1.042	49.042	64.164
0.053 190	0.13	0.00	1.293	49.293	64.293
0.056 199.8	0.15	0.08	1.425	49.425	64.425
MAX Ks	1.50	0.00	Total	Min	Max
Flow (m³/s)	Losses (m)		losses	head	head
, ,		fittings	(m)	(m)	(m)
0 0	0.00	0.00	0.000	48.000	63.000
0.003 10	0.00	0.00	0.006	48.006	63.006
0.006 20	0.00	0.00	0.022	48.022	63.022
0.008 30	0.01	0.00	0.048	48.048	63.048
0.011 40	0.01	0.00	0.084	48.084	63.084
0.014 50 0.017 60	0.02 0.02	0.00 0.01	0.131 0.188	48.131 48.188	63.131 63.188
0.017 60	0.02	0.01	0.166	48.255	63.255
0.019 70	0.03	0.01	0.233	48.333	63.333
0.025 90	0.05	0.02	0.420	48.420	63.420
0.028 100	0.06	0.02	0.518	48.518	63.518
0.031 110	0.08	0.02	0.626	48.626	63.626
0.033 120	0.09	0.03	0.745	48.745	63.745
0.036 130	0.11	0.03	0.873	48.873	63.873
0.039 140	0.12	0.04	1.012	49.012	64.012
0.042 150	0.14	0.04	1.161	49.161	64.161
0.044 160	0.16	0.05	1.320	49.320	64.320
0.047 170	0.18	0.06	1.490	49.490	64.490
0.050 180 0.053 190	0.20 0.23	0.06 0.07	1.669 1.859	49.669 49.859	64.669 64.859
0.056 199.8	0.23	0.07	2.055	50.055	65.055
DESIGN Ks	0.30	0.00	Total	Design	03.033
Flow (m³/s)	Losses (m)		losses	head	
, ,		fittings	(m)	(m)	
0.000 0	0.00	0.00	0.000	55.000	
0.003 10	0.00	0.00	0.005	55.005	
0.006 20	0.00	0.00	0.019	55.019	
0.008 30	0.01	0.00	0.040	55.040	
0.011 40	0.01	0.00	0.070	55.070	
0.014 50	0.01	0.00	0.108	55.108	
0.017 60	0.02	0.01	0.153	55.153	-
0.019 70 0.022 80	0.02 0.03	0.01 0.01	0.207 0.268	55.207 55.268	<b> </b>
0.022 80	0.03	0.01	0.266	55.338	<b> </b>
0.023 90	0.04	0.02	0.415	55.415	
0.031 110	0.06	0.02	0.501	55.501	
0.033 120	0.07	0.03	0.594	55.594	
0.036 130	0.08	0.03	0.695	55.695	
0.039 140	0.09	0.04	0.804	55.804	
0.042 150	0.10	0.04	0.921	55.921	
0.044 160	0.12	0.05	1.046	56.046	
0.047 170	0.13	0.06	1.179	56.179	
0.050 180	0.15	0.06	1.320	56.320	<b></b>
0.053 190	0.16	0.07	1.469	56.469	
0.056 199.8	0.18	0.08	1.622	56.622	l

# Calculation for System Curve of Flushing Water

### **Design Assumption**

Project

### 1 Used Code

- 1 Manual of Mainlaying Practice (2012 Edition)
- 2 Stomwater Drainage Manual (2018 Edition)

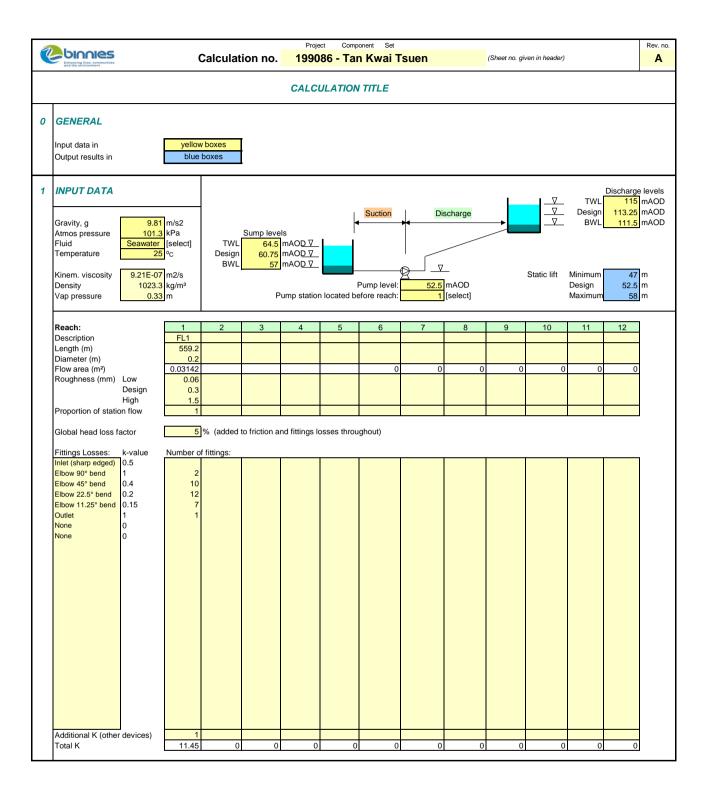
### 2 Design Assumption

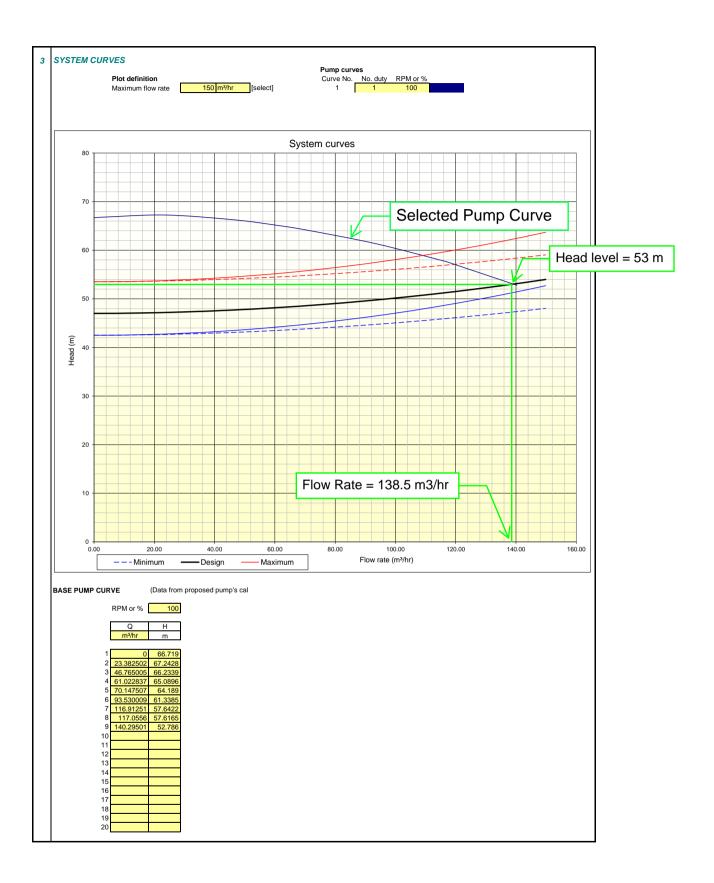
### FLUID PROPERTIES

	salinity	pressure (gauge)	density	kin visc	vapour pre	R	С	То	μο	То	Temp	μ, dyn visc
	(g/l)	(Pa)	(kg/m³)	(m2/s)	(m)	J/(kg.K)	(K)	(K)	(cP)	(oR)	(Kelvin)	(cP)
Water	0	n/a	997.05	0.00	0.33							
Air	n/a	101300	1.18	0.00	0.00	286.90	120.00	291.15	0.02	524.07	298.15	0.02

### 3 Used Equcation

- 1 Flow Rate = Cross-section Area x Average Speed
- 2 Velocity =  $\frac{Flow\ Rate}{Flow\ Area}$ ;
- 3 Velocity Head=  $\frac{Velocity^2}{2 \times Gravity}$ ;
- 4 Reynolds Number =  $\frac{Velocity \ x \ Gravity}{Kinem \ Viscosity}$ ;





	Fittings K Values	
	D	
	Item None	K 0
Inlet / outlet	Inlet (sharp edged)	0.5
	Outlet	1
	Inlet (re-entrant)	0.8
	Inlet (slightly rounded) Inlet (bellmouth)	0.25 0.05
Bends	Elbow 90° bend	1
	Elbow 45° bend	0.4
	Elbow 22.5° bend	0.2
	Elbow 11.25° bend Short R 90° bend	0.15 0.75
	Short R 45° bend	0.73
	Short R 22.5° bend	0.15
	Short R 11.25° bend	0.1
	Long R 90° bend Long R 45° bend	0.4 0.2
	Long R 22.5° bend	0.2
	Long R 11.25° bend	0.05
	Sweep 90°	0.2
	Sweep 45° Sweep 22.5°	0.1 0.05
	Mitre bend 90°	1.2
	Mitre bend 45°	0.4
	Mitre bend 22.5°	0.2
Transitions	Mitre bend 11.25°	0.15
Transitions	Taper up (4:5) Taper up (3:4)	0.03
	Taper up (1:2)	0.12
	Expansion 4:5	0.15
	Expansion 3:4	0.2
	Expansion 2:3 Expansion 1:2	0.35 0.6
	Expansion 1:3	0.8
	Expansion 1:5	1
	Contraction 5:4	0.15
	Contraction 4:3 Contraction 3:2	0.2 0.3
	Contraction 2:1	0.35
	Contraction 3:1	0.45
	Contraction ≥5:1	0.5
T-junctions	T line to branch 90°	1.2 0.6
	T line to branch 45° T line to branch 30°	0.6
	T straight through	0.35
	T line to branch 90° Radiused	0.8
Valves	Gate valve	0.12
	Butterfly valve Globe valve	0.3 5
	Plug valve	0.245
	Swing check valve	1
	Flap valve	1.5
User defined	Foot valve & strainer Other	2.5
Oser defined	Other	0
	Other Other	0
	Other	0
		( )

System Curve Calculation				
Gravity	9.81	m/s2		
Water visc	9.21E-07			
Density	1023.34	kg/m3		
U/s head	64.5	60.75	57 m	
D/s head	111.5	113.25	115 m	
Static lift	47	52.5	58 m	

Max flow		0.04167 m3/s
Section		1
Length	m	559.2
Diameter	m	0.2
Area	m2	0.03142
k		11.45
Flow factor		1

Flow factor			1	
MIN Ks			0.06	
Flow (m <sup>3</sup> /s)			Losses (m)	
1 10W (111 /3)				ings
	0.00	0.00	0.00	0.00
	0.00	7.50	0.02	0.00
	0.00	15.00	0.06	0.0
	0.01	22.50	0.13	0.02
	0.01	30.00	0.21	0.04
	0.01	37.50	0.32	0.06
	0.01	45.00	0.45	0.09
	0.01	52.50	0.59	0.13
	0.02	60.00	0.76	0.16
	0.02	67.50	0.95	0.2
	0.02	75.00	1.15	0.26
	0.02	82.50	1.38	0.3
	0.03	90.00	1.62	0.37
	0.03	97.50	1.88	0.43
	0.03	105.00	2.17	0.50
	0.03	112.50	2.47	0.58
	0.03	120.00	2.79	0.66
	0.04 0.04	127.50 135.00	3.13 3.49	0.74
	0.04	142.50	3.49	0.8
	0.04	149.85	4.25	1.02
MAX Ks	5.54	. 10.00	1.50	1.02
Flow (m³/s)			Losses (m)	
				ings
	0.00	0.00	0.00	0.00
	0.00	7.50	0.02	0.00
	0.00	15.00	0.09	0.0
	0.01	22.50	0.20	0.02
	0.01	30.00	0.36	0.04
	0.01	37.50	0.56	0.0
	0.01	45.00	0.80	0.09
	0.01	52.50	1.08	0.13
	0.02	60.00	1.41	0.10
	0.02	67.50	1.78	0.2
	0.02	75.00	2.19	0.26
	0.02 0.03	82.50 90.00	2.65 3.15	0.3
	0.03	97.50	3.69	0.3
	0.03	105.00	4.28	0.50
	0.03	112.50	4.91	0.58
	0.03	120.00	5.58	0.60
	0.04	127.50	6.30	0.74
	0.04	135.00	7.06	0.83
	0.04			
		142.50	7.86	0.93
	0.04	142.50 149.85	7.86 8.69	
DESIGN Ks				
DESIGN Ks Flow (m³/s)			8.69 0.30 Losses (m)	1.02
			8.69 0.30 Losses (m)	
	0.04	0.00	8.69 0.30 Losses (m) friction fitt 0.00	1.02 ings 0.00
	0.04 0.00 0.00	0.00 7.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02	1.02 ings 0.00 0.00
	0.04 0.00 0.00 0.00	0.00 7.50 15.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07	1.02 ings 0.00 0.00
	0.04 0.00 0.00 0.00 0.01	0.00 7.50 15.00 22.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14	1.02 ings 0.00 0.00 0.00
	0.04 0.00 0.00 0.00 0.01 0.01	0.00 7.50 15.00 22.50 30.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25	1.02 ings 0.00 0.00 0.02 0.02
	0.04 0.00 0.00 0.00 0.01 0.01 0.01	0.00 7.50 15.00 22.50 30.00 37.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38	1.02 ings 0.00 0.00 0.00 0.02 0.04 0.06
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01	0.00 7.50 15.00 22.50 30.00 37.50 45.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54	0.00 0.00 0.02 0.04 0.06 0.09
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73	1.02 ings 0.00 0.00 0.02 0.04 0.00 0.09 0.13
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.02	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94	1.02 ings 0.00 0.00 0.02 0.04 0.00 0.13
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.02 0.02	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00 67.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18	1.02 ings 0.00 0.00 0.02 0.00 0.01 0.11 0.12
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.02 0.02 0.02	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00 67.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18	1.02 ings 0.00 0.00 0.02 0.04 0.05 0.05 0.11 0.11 0.22
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02	0.00 7.50 15.00 22.50 30.00 52.50 60.00 67.50 82.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75	1.02 ings 0.00 0.00 0.02 0.04 0.09 0.13 0.14 0.22 0.26
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.03	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00 67.50 75.00 82.50 90.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18	1.02 ings 0.00 0.00 0.02 0.04 0.01 0.11 0.22 0.26 0.33
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02	0.00 7.50 15.00 22.50 30.00 52.50 60.00 67.50 82.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07	1.02 ings 0.00 0.00 0.02 0.04 0.01 0.11 0.22 0.33 0.33
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.01 0.02 0.02 0.02 0.02 0.03 0.03	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00 67.50 75.00 82.50 90.00 97.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07	1.02 ings 0.00 0.00 0.00 0.00 0.01 0.11 0.11 0.22 0.22
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02 0.03 0.03	0.00 7.50 15.00 22.50 30.00 37.50 60.00 67.50 90.00 97.50 105.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07 2.42 2.80	1.02 ings 0.00 0.00 0.00 0.00 0.01 0.11 0.11 0.22 0.33 0.33 0.44 0.56
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02 0.03 0.03	0.00 7.50 15.00 37.50 45.00 67.50 60.00 67.50 90.00 97.50 105.00 112.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07 2.42 2.80 3.21	1.02 ings 0.00 0.00 0.02 0.04 0.33 0.43 0.55 0.66
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02 0.03 0.03 0.03	0.00 7.50 15.00 37.50 45.00 67.50 60.00 67.50 90.00 97.50 105.00 112.50 120.00	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07 2.42 2.80 3.21 3.64	1.03 ings 0.00 0.00 0.00 0.00 0.00 0.01 0.11 0.1
	0.04 0.00 0.00 0.00 0.01 0.01 0.01 0.02 0.02 0.02 0.02 0.03 0.03 0.03 0.03	0.00 7.50 15.00 22.50 30.00 37.50 45.00 52.50 60.00 67.50 75.00 82.50 90.00 97.50 105.00 112.50 120.00 127.50	8.69 0.30 Losses (m) friction fitt 0.00 0.02 0.07 0.14 0.25 0.38 0.54 0.73 0.94 1.18 1.45 1.75 2.07 2.42 2.80 3.21 3.64 4.10	1.02 ings 0.00 0.00 0.02 0.02 0.04

Total	Min	Max
losses	head	head
(m)	(m)	(m)
0.00	42.50	53.50
0.02	42.52	53.52
0.07	42.57	53.57
0.16	42.66	53.66
0.27	42.77	53.77
0.40	42.90	53.90
0.57	43.07	54.07
0.76 0.97	43.26	54.26 54.47
1.21	43.47 43.71	
1.48		54.71 54.98
1.40	43.98 44.27	55.27
2.09	44.59	55.59
2.43	44.93	55.93
2.80	45.30	56.30
3.20	45.70	56.70
3.62	46.12	57.12
4.06	46.56	57.56
4.53	47.03	58.03
5.03	47.53	58.53
5.54	48.04	59.04
Total	Min	Max
losses	head	head
(m)	(m)	(m)
0.00	42.50	53.50
0.03	42.53	53.53
0.11	42.61	53.61
0.24	42.74	53.74
0.42	42.92	53.92
0.65	43.15	54.15
0.93	43.43	54.43
1.27	43.77	54.77
1.65	44.15	55.15
2.09	44.59	55.59
2.57	45.07	56.07
3.11	45.61	56.61
3.70	46.20	57.20
4.33	46.83	57.83
5.02	47.52	58.52
5.76	48.26	59.26
6.55	49.05	60.05
7.39	49.89	60.89
8.28	50.78	61.78
9.22	51.72	62.72
10.20	52.70	63.70
Total	Design	
losses	head	
(m)	(m)	
0.00	47.00	
0.02	47.02	
0.08	47.08	
0.18	47.18	
0.31	47.31	
0.47	47.47	
0.67	47.67	
0.90	47.90	
1.16	48.16	
1.46	48.46	
1.79	48.79	
2.16	49.16	
2.56	49.56	
3.00	50.00	
3.47	50.47	
3.97	50.97	
4.51	51.51	
5.08	52.08	
5.69	52.69	
6.33	53.33	
6.99	53.99	